



BUILDBLOCK BUILDING SYSTEMS

BUILDDECK ROOF & FLOOR DECKING SYSTEM DESIGN, ENGINEERING, AND INSTALLATION MANUAL

REVISED MAY 2016



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DISCLAIMER

Though every reasonable effort has been made to ensure the accuracy and relevance of the information contained in this design manual, BuildBlock Building Systems, LLC, our partners and affiliates assume no responsibility or liability for damages, failure or otherwise adverse results related to or resulting from information contained herein.

All tables, charts, pictures, descriptions and/or any other components depicted in this manual are intended to be used for estimation purposes only, and in no way are intended to be, or shall be interpreted as construction plans or approved Engineered documents.

All final design of concrete, reinforcement, shoring, system elements and interfaces are the responsibility of the project specific Engineer of Record for each application.

This system is designed to be used with the design advice and oversight of a professional trained in structural design and engineering.

Elements of this Manual has been reviewed for accuracy by:

Advanced Structural Engineering II 1265 South Semoran Boulevard, Suite 250 Winter Park, FL 32792

Long Span Tables Developed by:

McLaren Engineering Group 5728 Major Boulevard, Suite 603 Orlando, FL 32819

INTRODUCTION

PREFACE

This version of the BuildDeck Installation Manual was originally published in July 2015. Changes to this document, however, may occur without notice and users should contact BuildBlock Building Systems, LLC for the most current printed or downloadable version at buildblock.com. It is the purchaser's and/or contractor's responsibility to always use the most current and up-to-date version of the installation manual when installing BuildDeck Roof & Floor Decking System panels and/or products.

This manual was designed to be used as a reference guide only. All figures, calculations, designs, drawings, pictures and references thereto are provided as examples. All installations must be designed and approved by the project specific Engineer of Record. This manual is not intended to be used as a replacement or substitute for the actual training by an experienced and trained BuildBlock building professional. Before starting any project, BuildBlock recommends that you receive proper training. BuildBlock also recommends that you consult with a design professional trained in the discipline and familiar with the type and scope of project to be built. Training is available by contacting BuildBlock Building Systems, LLC at buildblock.com or 866-222-2575.

BuildBlock Building Systems, LLC believes the information contained herein to be accurate at the time of preparation and publication. The information has been compiled using sources believed to be reliable and accurate. Neither BuildBlock Building Systems, LLC, nor its employees or representatives make any representation or warranty, either expressed or implied, whether arising by statute, operation of law, custom of trade or otherwise, with respect to the accuracy or completeness of the information contained in this document or its fitness for any particular purpose, nor do they assume liability for damages or injury resulting from the application of such information.

BuildBlock Building Systems, LLC assumes no responsibility regarding the use of its products or any other third party products referred to in this document. It is the full responsibility of the user to comply with all applicable regulations and building code requirements concerning the use of these products and any other products outlined in this product manual. It is further the responsibility of the user to research and understand safe methods of use and handling of these products. To properly comply with the building codes in your area, contact your local distributor, dealer, or building code inspector.

PRODUCT WARNINGS

Many new types of treated wood products using ACQ (alkaline copper quaternary) are highly corrosive to metal components. BuildBlock Building Systems, LLC recommends that any metal products or components should not be used in contact with these treated lumber products unless you ensure the compatibility of your treated lumber with the metal components. Please consult with your project engineer to specify the type and sizing of all corrosion resistant metal connectors, anchor bolts, fasteners or other metal components. Please note that metal connectors, anchor bolts, fasteners or other metal components will corrode

and lose their load carrying capacity, if installed in corrosive environments.

TRADEMARKS

BuildBlock or BB BuildBlock and BuildDeck and any other drawings, symbols or marks identifying products and/or services of BuildBlock Building Systems, LLC are registered trademarks of BuildBlock Building Systems, LLC. All other trademarks, drawings, symbols or marks are the property of their respective owners.

INTRODUCTION

The BuildDeck Roof & Floor Decking System was designed to provide a cost effective, safe and easy to use deck system for the purpose of creating intermediate floors as well as pitched and flat roofs. Further plans include design, testing and engineering guidelines for the use of the BuildDeck system as a cost effective alternative to traditional tilt-up wall systems.

This system cuts costs by implementing light weight structural steel C channels as opposed to the large structural steel studs common in other systems. We have accomplished this by using high quality EPS foam molded at higher densities than competing systems. In testing, one panel was loaded to over 1000 lbs. without failure. Additionally the steel C channels provide a positive connection between the concrete and the ceiling element increasing the safety of the tenants in the structure.

The BuildDeck panels are designed to be molded in the same facilities as BuildBlock ICFs. This allows product to be shipped on the same truck to further drive down construction and shipping costs. They are packaged in bundles similar in size to our other products to simplify storage and handling.

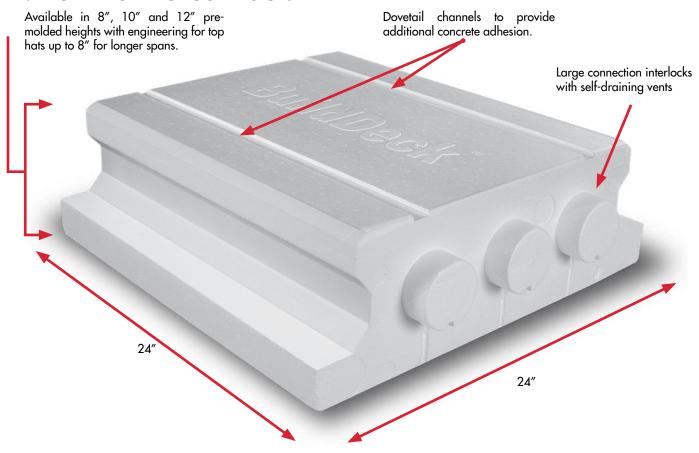
We have worked very hard to provide a system that adds value to your project and your business. BuildBlock will always strive to develop this and other systems to provide the most benefit for our clients and their customers. We welcome your comments and look forward to better understand how the BuildDeck system promotes the growth of your business.

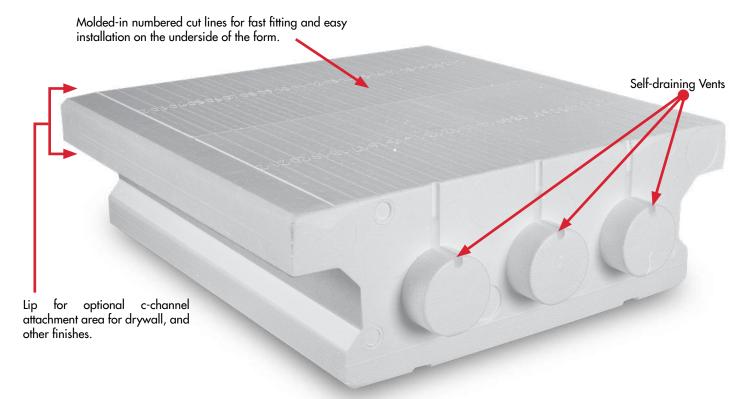
WARRANTY

BuildBlock warrants our products to be free of manufacturing defects. In the event that a defect occurs, BuildBlock will remedy the occurrence as per the guidelines of the BuildBlock Warranty Policy available on our website at buildblock.com.

SECTION 1: PRODUCT FEATURES

1.1 BUILDDECK PRODUCT DESIGN





1.2 PRODUCT SPECIFICATIONS OVERVIEW

BuildDeck Roof & Floor Decking panels are 24" x 24", modular panels that are connected together and placed aside one another to create a stay in place form for deck beams and concrete caps (slabs). The forms stay in place to provide interior attachment points, insulation and noise mitigation.

SECTION 2: RECOMMENDED TOOLS AND ACCESSORIES

BuildBlock Building System carries a full line of ICF construction accessories that complement BuildBlock ICFs and related products when building residential homes, commercial, and industrial, buildings. Ensuring you have the tools to make installation quick and efficient will decrease frustration and save you money and time, increasing your bottom line.

- Folding Pruning Saw
- Skill saw
- Keyhole saw
- Table saw (optional, for convenience)
- Hammer drill, cordless drill
- Rebar tie tool
- Hot Knife or Hot Knife Kit combo*
- Hammer
- Framing square
- Concrete trowel
- Level
- Tape measure
- Transit or laser level
- Mason's line and chalk line
- Rebar bender
- Rebar cutter
- Wall alignment (bracing) system*
- Scaffold planks
- Concrete pencil vibrator, 3/4" low impact 1" maximum
- Foam guns, foam, and foam cleaner*
- Work gloves
- Sun Screen
- Broom and floor scraper

See the full BuildBlock Installation and Technical Manual for more products and accessories. Visit buildblock.com for more information.

SECTION 3: ESTIMATING BUILDDECK

3.1 ESTIMATING BUILDDECK PANELS

BuildDeck panels are modular deck panels. Each panel, regardless of height will cover four square feet of floor space. The formula used to determine the number of panels required will be length times width, divided by four (Area [in ft.] / 4).



Figure 3.1.1 BuildDeck material estimating diagram using area of a rectangle divided by area of product.

EXAMPLE

 $L \times W / 4 = [Total \ Number \ of \ Required \ Panels]$

Keep in mind that cut sections of a BuildDeck panel may be used. For instance, if your span is 20'-6'' long, you will need five full panels, and $\frac{1}{4}$ (6 inches) of a sixth panel. For this reason you are able to cut one panel into four sections to complete your span, and lower the total number of panels required to complete the floor or roof.

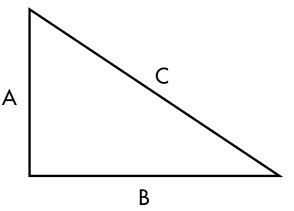


Figure 3.1.2 BuildDeck pitched roof material uses the rise of the roof as the length in the area calculation.

Calculation: A²+B²=C²

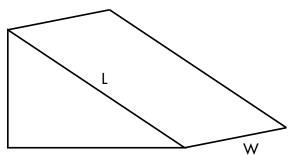


Figure 3.1.3 Busing the rise of the roof as the length calculate the area of the roof, then divide by the area of the product to determine amount of material needed.

When calculating panels for a pitched roof, ensure that the number used for the length is representative of the hypotenuse of the triangle, not the base width of the structure.

This number should be used as the length then multiplied by the width of the roof section and then divided by the area of the product in square feet to determine the amount of product needed.

3.2 ESTIMATING CONCRETE

Use the following table to calculate the volume of concrete needed per BuildDeck panel used.

BuildBlock recommends 4000 psi concrete for use on decking systems, but this will ultimately be determined by the Engineer of Record and should not be assumed.

BUILDDECK CONCRETE ESTIMATION (PER PANEL)					
HEIGHT	LENGTH	WIDTH	AREA	CONCRETE (YD3)	
BD-800 (8")	24"	24"	4 ft ²	* .05111111	
BD-1000 (10")	24"	24"	4 ft ²	* .05851852	
BD-1200 (12")	24"	24"	4 ft ²	* .06037037	
BD-1200+2 (14")	24"	24"	4 ft ²		
BD-1200+4 (16")	24"	24"	4 ft ²		
BD-1200+6 (18")	24"	24"	4 ft ²		
BD-1200+8 (20")	24"	24"	4 ft ²		

^{*} Concrete volume is based on a top cap thickness of 3". Add .0122222 cu. yd. per form for each additional 1".

3.3 ESTIMATING REBAR

Each BuildDeck project will vary in terms of steel per the design specified by the Engineer of Record specific to the project. For this reason, calculations will be based on their recommendations.

As a general rule of thumb, each beam will have (2) runs of rebar in the bottom, and each concrete top cap or slab will contain a 12" x 12" grid of rebar, more than likely with #4 steel.

For estimation purposes:

- 5 feet of steel per BuildDeck panel used will provide an approximation of beam steel needed.
- 9 feet of steel per BuildDeck panel used will provide an

approximation of grid steel needed.

NOTE: The beam steel and grid steel will more than likely be different sizes so take this into consideration.

3.4 ESTIMATING BUILDDECK SHORING

Many methods of shoring may function to support the BuildDeck Flooring System during construction and curing. BuildBlock has commissioned the design of a cost effective, Engineer approved system that we recommend. Regardless of the system you choose, it is the responsibility of the user and/or contractor to ensure that all codes, laws and construction methods pertinent to your particular build are followed in regard to safety and structural integrity.

RECOMMENDED BUILDDECK SHORING DESIGN

The recommended BuildDeck shoring system is a wood framed shoring system comprised of stud walls spaced at 6ft o.c., with joists spanning underneath the BuildDeck panels at 2ft o.c.. The joists are to be centered along the joint between panels, perpendicular to the beam section of the form. The stud framed walls will be placed perpendicular to the joists, and parallel to the beams. The materials for shoring may be reused to frame interior walls in the rest of the project.

HIGH LOAD/ REUSABLE BUILDDECK SHORING DESIGN

An alternative BuildDeck shoring system consists of 2×8 girders supported every 72 inches by 4×4 , 4×6 or metal posts running parallel to the concrete beams being formed, and 2×6 joists hung from these girders every 24 inches O.C. from Simpson or other code approved joist hangers to support the panels laterally.

This system has been reviewed and approved by a Professional Engineer as a medium duty shoring system. Most components can be re-used for future BuildDeck project installs.

 2×8 Girders – Width of floor (Length [in feet] of wall perpendicular to beams) divided by six, multiplied by length of wall parallel to beams, multiplied by two. This will give you the number of linear feet of 2×8 board required. Divide this number by twelve to get the amount of 12' 2x8 boards needed.

Formula: W/6 x L x 2

 2×6 Joists - Length of floor divided by two, multiplied by width of floor equals linear feet of 2×6 board required.

Formula: L/2 x W

NOTE: We recommend that you build girders in 12' and 6' lengths as the supporting posts are designed to be placed every 6'. Additional lengths can be added to an end by splicing girders with longer spacers. Many systems are commercially available to extend post heights. Check the Ellis Manufacturing and other web sites for options.

3.5 ENGINEERING STATEMENTS

The following engineering statements from McLaren Engineering Group detail the installation of both BuildDeck shoring systems.



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May 3, 2016

BuildBlock Building Systems, LLC 9701 N. Broadway Extension Oklahoma City, OK 73114

Attn: Mr. Mark Kerfoot

Re: BuildBlock BuildDeck Shoring System for

BuildDeck BD-800, BD-1000, and BD-1200

McLaren File No. 150609.00

Dear Mr. Kerfoot:

At the request of BuildBlock Building Systems, LLC (BuildBlock), McLaren Engineering Group (McLaren) designed a wood framed shoring system for temporary support of BuildDeck Sizes BD-800, BD-1000, and BD-1200 with up to an 8" expanded polystyrene riser and up to a 4" thick concrete deck. All lumber used with the shoring system is to be Southern Yellow Pine No. 2 or better. The shoring system can accommodate ceiling heights up to 12'-0" and BuildDeck Spans between 12'-0" and 40'-0" consistent with the span capabilities of the three listed BuildDeck sizes. The system consists of framed walls with 2x4 or 2x6 studs at 16" o.c and 2x8 or 2x10 joists at 24" o.c on top of these walls. In-plane 2x4 wall bracing and out-plane 2x4 wall bracing provide lateral stability for the shoring system and shoring system attachment to the surrounding insulated concrete form walls is not required. See the attached Figure 1 for a view of the described shoring system and for member fasteners. Additional walls may be added and wall length may be increased to accommodate varying deck dimensions.

The wood framed shoring system is intended for temporary use during construction for a period of six weeks or less to support the indicated BuildDeck systems during installation and concrete curing. Review of the concrete slab or substrate supporting the shoring system was outside of the scope of this analysis and is by others. The following construction loads were used in the structural design of the shoring system:

Maximum Dead Load of BuildDeck and Concrete: 97 psf Construction Live Load: 25 psf

Construction Wind Load (Per ASCE 37-02): $0.75 \times 90 \text{ mph} = 67.5 \text{ mph} \text{ (ASD)}$

Construction live load is to be light duty loading i.e. during concrete placement on the BuildDeck, the deck should be sparsely populated with personnel, hand-operated

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5728 Major Blvd, Suite 603 Orlando, FL 32819 Phone (407) 354-5411 Fax (407) 354-3466 e-mail: fl@mgmclaren.com On the web: www.mgmclaren.com BuildBlock BuildDeck Shoring System McLaren File No. 150609.00

Page 2 May 3, 2016

equipment, and staging of materials for lightweight construction. When building the shoring system, duplex nails of the size indicated in Figure 1 may be used for ease of shoring disassembly. At the contractor's option, wood screws may be substituted for nails as follows:

Nail Size Shown in Figure 1 10d Common Nail 16d Common Nail

Wood Screw Substitute #12 Wood Screw x 3" Long #14 Wood Screw x 3½" Long

No additional gravity loads in excess of the total 122 psf dead load plus live load are allowed on the wood framed shoring system prior to the concrete curing. It is recommended to leave the BuildDeck shoring system in place for the full 28 day cure time of the concrete unless early high strength concrete has been specified or other analysis has proven the BuildDeck system will not be damaged or compromised by removing the shoring early. If constructed as detailed in the attached Figure 1, the BuildDeck Shoring System will be adequate for temporary support of BuildDeck Sizes BD-800, BD-1000, and BD-1200 with up to an 8" expanded polystyrene riser and up to a 4" thick concrete deck. The seal show below is for responsibility for the temporary wood framed shoring system described in this letter and detailed in the attached Figure 1, and is not for responsibility of the final constructed structure, including the BuildDeck system supported by the shoring.

Feel free to call our office with any questions.

Regards,

The Office of McLaren Engineering Group d/b/a McLaren Technical Services, Inc.



Andrew Habel, P.E. #69819 Florida Division Manager

cc: ACH, Internal File 160003.00

Attachments: Figure 1: BuildBlock BuildDeck Shoring System for BuildDeck BD-800, BD-

1000, and BD-1200 with up to an 8" expanded polystyrene riser and up to

a 4" thick concrete deck.



MCLAREN TECHNICAL SERVICES, INC.

BuildBlock BuildDeck Shoring System McLaren File No. 150609.00

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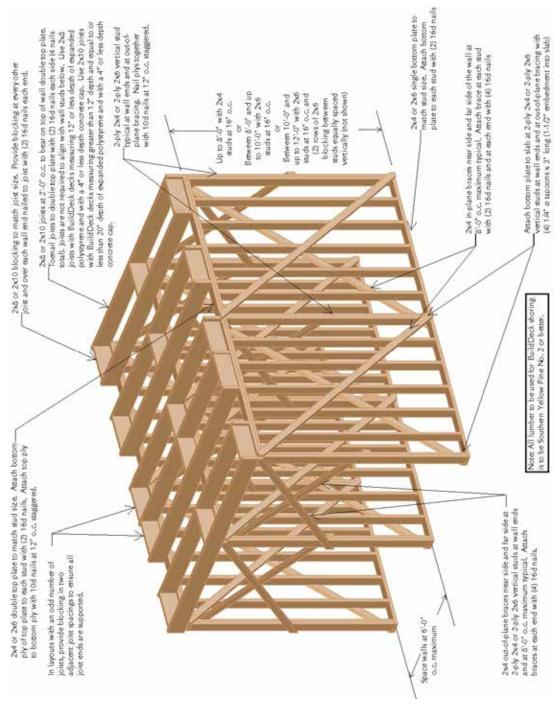


Figure 1: BuildBlock BuildDeck Shoring System for BuildDeck BD-800, BD-1000, and BD-1200 with up to an 8" expanded polystyrene riser and up to a 4" thick concrete deck.



MCLAREN TECHNICAL SERVICES, INC.

PAGE 7



September 22, 2009

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BuildBlock Building Systems, LLC 9701 N. Broadway Extension Oklahoma City, OK 73114

Attn: Mr. Justin Wallace

Re: BuildBlock BuildDeck Shoring System

MEG File: 109735.00

Dear Mr. Wallace:

This letter is presented as McLaren Engineering Group's analysis of the proposed shoring system of the BuildDeck floor system. The BuildDeck floor system consists of 2' x 2' EPS panels with concrete poured on top and in between two adjacent panels, forming a web and a flange. The layout of the shoring system was proposed by BuildBlock and consists of the following: wooden 2x or 4x posts spaced at 6' on center in both directions that support 2x girders that support 2x joists spaced at 2' on center. Some girders may be double-ply. Blocking in between these plies may range from ½" thick plywood to 1-1/2" thick 2x material; whatever is needed to allow for the connection to the post. McLaren understands that there are three sizes of BuildDeck EPS panels that may be used; the panel that requires the most volume of concrete has been used for design.

The following are descriptions of the different abbreviations used in this letter.

SYP – Southern Yellow Pine DFL (N) – Douglas Fir-Larch (North) or Douglas Fir-Larch

L.L. - Live Load

Table 1 summarizes the allowable live load for the four various shoring framing systems.

JOIST TYPE	GIRDER TYPE	INTERIOR POST	EXTERIOR POST	ALLOWABLE L.L.
2X6 SYP #2	2X8 SYP #2	4X4 SYP STANDARD ² ³	2X6X10' SYP #21	50 psf
2X6 DFL (N) #24	2X8 DFL (N) #24	4X4 DFL (N) #2 ^{2 3 4}	2X6X10' DFL (N) #24	32 psf ⁴
2X8 SYP #2	2X10 SYP #2	4X4 SYP STANDARD ² ³	4X4 SYP STANDARD ² ³	90 psf
2X8 DFL (N) #2	2X10 DFL (N) #2	4X4 DFL (N) #2 ² 3	4X4 DFL (N) #2 ^{2 3}	79 psf

Table 1 - Allowable Loads

Table 1 Notes:

- a Note that post height doesn't include height of any jacking system employed.
- 1 2x6x10' post must be braced in the weak axis at mid-height. An acceptable bracing practice would be to screw the post to the ICF wall at 12" on center. If post height is 8' or less, acceptable post is as shown or 2x8x8' unbraced.
- 2 4X6 post may be substituted for 4x4 post.
- 3 Construction, standard, and #2 grades are interchangeable if same species is used.
- 4 This system that employs 2x6 DFL (N) #2 joists may only be used where construction type is explicitly light duty.

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Table 2 summarizes the required Simpson (or other) connectors to be used.

MEMBERS	REQUIRED LOAD	CONNECTOR	AVAILABLE LOAD
2X6 JOIST TO 2X8 GIRDER	684 lb	LUS26 W/ (4) 10d NAILS	830 lb
2X8 JOIST TO 2X10 GIRDER	930 lb	LUS28 W/ (6) 10d NAILS	1055 lb
BEAM TO 4X COLUMN	STABILITY	(2) LPC4Z1 OR ELLIS CAP	STABILITY
BEAM TO 2X6 COLUMN	STABILITY	LPC6Z ¹	STABILITY
GIRDER TO 2X4 BLOCKS	STABILITY	(4) 16d NAILS	STABILITY

Table 2 - Required Connectors

Table 2 Notes:

- a Values have been taken from the 2009-2010 Simpson Wood Construction Connectors catalog.
- 1 Use (8) 10d nails into the beam and (8) 10d nails into the post.

The following are recommendations that must be followed to ensure that the shoring system functions properly:

- The shoring system must be somehow braced laterally in the direction parallel to the joist span. The system is considered braced if exterior posts are screwed to an ICF wall.
- All instructions and precautions for Simpson connectors and Ellis screw jacks and post caps must be followed.
- 3) All joists must be snug fit against the joist hangar on all four sides of contact.

Included below are pages 11, 19 and 20 from SEI/ASCE37-02 which is a standard for the design loads on structures during construction. Page 11 includes definitions and the table on page 19 classifies construction into four different types: very light duty, light duty, medium duty, and heavy duty. Each type has an associated uniform load which does not include dead load, construction dead load, or fixed material loads, so this uniform load can be considered a live load in the BuildDeck application. There are descriptions of these four types of construction to the right of the table and onto page 20. Two characteristics of heavy duty construction are concrete transport and placement using motorized buggies and material storage.

Note that in no case should heavy duty construction take place on the proposed shoring system; however it is acceptable to store limited quantities of 20 gage sheet metal and EPS panels over the girders. Any storage over the joists should be kept very minimal at all times.

It is the opinion of McLaren that the main construction type for the BuildDeck application is medium duty and therefore most systems require the use of a live load of 50 psf. Therefore, the shoring framing system employing 2x6 DFL (N) #2 joists should not be used unless construction type is explicitly light duty, which may be the case in some shoring applications.



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STANDARD

4.0 CONSTRUCTION LOADS

4.1 General Requirements

The provisions of this section shall be used to define the construction loads for the design of both temporary structures and permanent structures subject to loads during construction. These loads are to be combined with other applicable loads per the requirements of Section 2.

When a construction loading is covered in another document that is acceptable to the authority having jurisdiction and written to address a specific material or method of construction, the more applicable document shall be permitted to be followed.

Stairs, ladders, and elevators are not addressed in this standard.

4.1.1 Definitions

Construction loads: those loads imposed on a partially completed or temporary structure during and as a result of the construction process. Construction loads include, but are not limited to, materials, personnel, and equipment imposed on the temporary or permanent structure during the construction process.

Construction dead load, C_D: the dead load of temporary structures that are in place at the stage of construction being considered. The dead load of the permanent structure, either partially complete or complete, is not included in C_D; the dead load of the permanent structure is defined as dead load, D, in Section 3.1.

Individual personnel load: a concentrated load of 250 lb (1.1 kN) that includes the weight of one person plus equipment carried by the person or equipment that can be readily picked up by a single person without assistance.

Working surfaces: floors, decks, or platforms of temporary or partially completed structures which are or are expected to be subjected to construction loads during construction.

4.2. Material Loads

The material dead loads consist of two categories:

- 1. fixed material loads (FML)
- 2. variable material loads (VML)

The FML is the load from materials that is fixed in magnitude. The VML is the load from materials that varies in magnitude during the construction process. If the local magnitude of a material load varies during the

SEI/ASCE 37-02

COMMENTARY

C4.0 CONSTRUCTION LOADS

C4.1 General Requirements

The loads for some temporary structures, such as those that retain lateral pressures of earth, are not defined in Section 4; refer to Section 5 for lateral pressures of earth.

Standards and other documents applicable to specific materials or methods of construction have been developed and are recognized and used extensively.

C4.2. Material Loads

This section separates material dead loads into two categories: FML and VML, which are separated to permit the use of an appropriate load factor for each category in strength design. This approach recognizes the difference in the variability of the load between the two categories.

This section addresses the loads from materials and is not intended to apply to equipment loads. Per-

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SEI / ASC E 37-02

SEI/ASCE 37-02

STANDARD

4.8 Application of Loads

4.8.1 Combined Loads

The design construction load shall include the critical combination of personnel, equipment, and material loads.

4.8.1.1 Working Surfaces. Structures supporting working surfaces as defined in Section 4.1 shall be designed for the combined material, personnel, equipment, and other applicable construction loads.

When the construction operation fits the definition in Table 2, the designer is permitted to design for the tabulated uniform loads as the vertical load from the combination of personnel, equipment, and material in transit or staging. When the construction operation does not fit the definitions in Table 2, the design shall be for the actual loads. Concentrated loads shall be considered separately.

Table 2 Classes of Working Surfaces for Combined Uniformly Distributed Loads

Operational Class	Uniform Load µnf (kN/m²)
Very light duty: sparsely populated with personnel; hand tools; very small amounts of construction materials	20 (0.96)
Light duty: sparsely populated with personnel; hand operated equipment; staging of materials for lightweight construction	25 (1.20)
Medium duty: concentrations of personnel; staging of materials for average construction	50 (2.40)
Heavy duty: material placement by motorized buggies; staging of materials for heavy construction	75 (3.59)

^{*} Loads do not include dead load, D; construction dead load, $C_{\rm b}$; or fixed material loads, $C_{\rm max}$.

COMMENTARY

C4.8 Application of Loads

Construction loads depend very much on the specific planning and processes of construction. This section includes rules for applying and combining the various loads, as well as traditional minimums for several common construction processes.

C4.8.1 Combined Loads

The combination of the various forms of construction loads, materials, personnel, and equipment is an important step in engineering for construction, requiring careful application of professional judgment.

C4.8.1.1 Working Surfaces. It is traditional to design many working surfaces for a uniformly distributed load that is meant to include all construction loads, except for materials in their final position.

Temporary structures have often been designed, advertised, and specified by the light, medium, and heavy duty ratings given in Table 2. This standard also applies to partially completed structures, and the same terminology is adopted. Different styles of construction and different segments of the construction industry have different traditions for design loads on partially completed structures during construction, and this section of the standard is an attempt to unify the industry on a common basis.

Examples of construction operations that have traditionally been designed for the loads given in the table are

Very light duty:

Roofing, reroofing, excepting situations with stockpiles of ballast

Access catwalks

Painting, caulking

Maintenance using hand tools

Light duty:

Light frame construction

Concrete transport and placement by hose and concrete funishing with hand tools

Medium duty

Concrete transport and placement by buckets, chutes, or handcarts

Concrete finishing using motorized screeds

Masonry construction with tile or hollow lightweight concrete units

Structural steel erection or concrete reinforcing steel placement

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DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION

STANDARD

COMMENTARY

Heavy duty:

Concrete transport and placement using motorized buggies

Masonry of brick or heavy-weight concrete units Material storage

Conflicts between provisions of this section and those in ASCE 3-91 and ASCE 9-91 are acknowledged.

Following are examples of working surfaces that do not fall under Table 2:

Roofs for which design is controlled by building code live load or snow loads that are less than values in Table 2.

Attics or hung ceilings that provide access for maintenance, installation of utilities, and emergency services such as firefighters.

These working surfaces must be addressed in accordance with Sections 4.8.1.1 and 4.8.4.

C4.8.1.2 Specification of Temporary Structures. This requirement will encourage uniformity in terminology for capacity of scaffolds and similar structures.

4.8.2 Partial Loading

loads shall be as given in Table 2.

The full intensity of the construction load applied only to a portion of the length of a structure or member shall be considered if it produces a more unfavorable effect than the same intensity applied over the full length of the structure or member.

4.8.1.2 Specification of Temporary Structures. When

temporary structures are specified by load name, the names of the load class and the magnitude of design

4.8.3 Reduction in Construction Loads

4.8.3.1 Material Loads. No reduction is allowed for fixed or variable material loads, except to the extent that small amounts of material in transit or staging are included in uniformly distributed personnel, equipment, and material loads, such as those in Table 2.

4.8.3.2 Personnel and Equipment Loads. When justified by an analysis of the construction operations, members having an influence area of 400 ft² (37.16 m²) or more may be designed for a reduced uniformly distributed personnel and equipment load determined by applying the following formula:

$$C_P = L_{tr} (0.25 + 15/\sqrt{A_f})$$
 (4-6)

$$C_{PSI} = L_0 (0.25 + 4.57/\sqrt{A_I})$$
 (4-6 SD)

C4.8.2 Partial Loading

Partial-length loads on a beam or truss may produce higher shear on a portion of the span than a fulllength load. Checkerboard loadings on floors and multistory frames produce the highest positive and negative moments. Cantilevers cannot rely on a possible construction load on the anchor span for equilibrium. ASCE 7-95 describes other possible conditions of designing members or floors for partial loading.

C4.8.3.2 Personnel and Equipment Loads. Uniformly distributed loads are a convenient substitute for computing the combined effect of several concentrated loads. As such they are generally calibrated to a particular area. For smaller areas, the concentrated loads control structural design. The nature of transient concentrated loads, such as personnel and equipment, is that their spacing is not uniform, thus, for areas larger than the calibration area, the uniform load may be unnecessarily conservative.

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If you have any questions and or comments please contact me at (407) 354-5411 or via email at roleck@mgmclaren.com.

Very truly yours The Office of McLaren Engineering Group M.G. Mclaren, P.C.

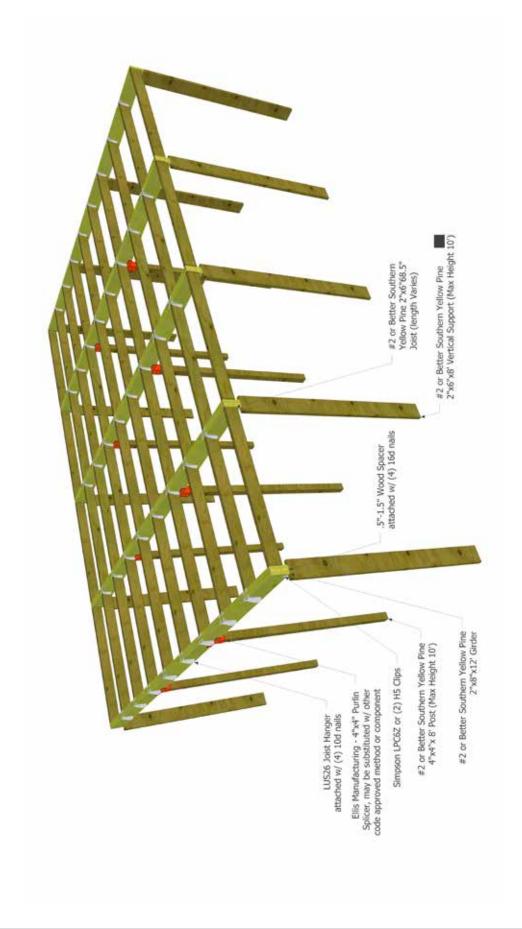


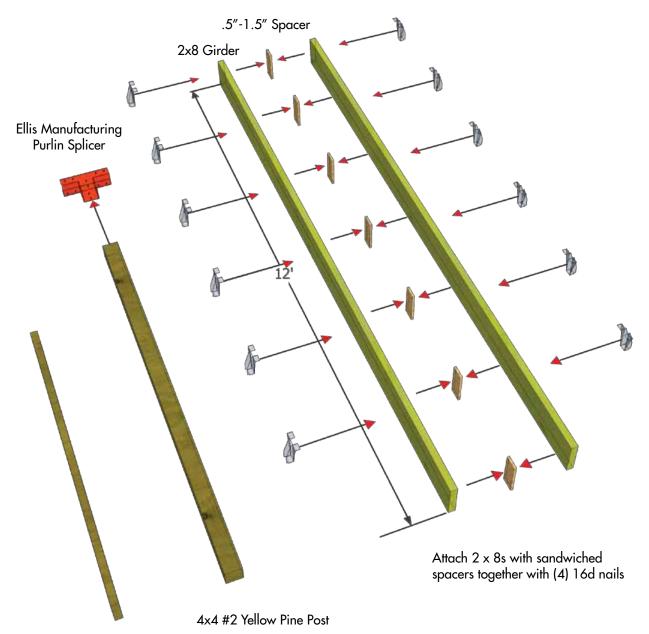
Robert F. Oleck, Ph.D., P.E. Regional Director

cc: MGM, WRM, DWH,

P:\PROJ109\109735-BUILDBLOCK FLOORDECK SHORING\3. CORRESPONDENCE\LETTERS\SENT\LTR01-J.WALLACE-109735-092209.DOC



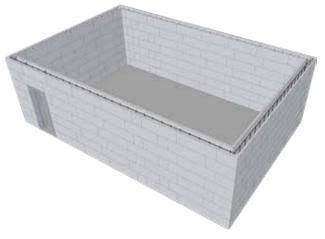




2 x 6 Joist, 68.5" Typical length may vary based on project, end row of joists will be 68.75" in length.

Figure 3.5.1 BuildDeck shoring girder assembly.

SECTION 4: BUILDDECK INSTALLATION

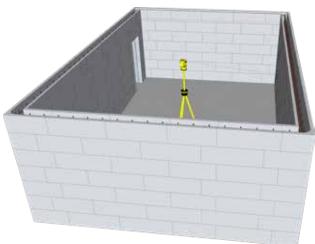


4.1 WALL INSTALLATION

Stack BuildBlock ICF Forms to the intended "top of floor height". Decide if you will top mount or side mount the BuildDeck system to your ICF or other wall system. This will determine the height you will cut the interior panel of the wall.

Note that steel frequency may be increased in walls to support heavier loads as specified by the Engineer or construction plans/guidelines.

Provide appropriate steel end length stub out from wall if above story is to be constructed in ICF.



4.2 INTERIOR WALL PANEL HEIGHT

After determining the mounting method for your floor system, use a laser level or construction level to mark the intended "bottom of floor" height.

Bottom of Floor is determined by adding the depth of the deck system being used and the depth of the intended concrete cap. Subtract the sum from the overall height of the wall (top of floor height).



4.3 CUT INTERIOR PANEL

Once the ceiling or "bottom of floor" height is determined, cut the interior wall panel to the appropriate height.

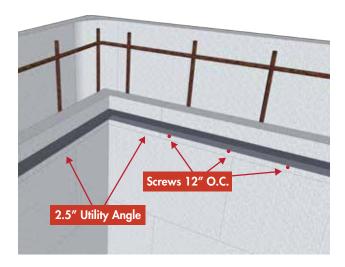
The interior panel height may differ based on project design, needs or preferred installation practices. The goal is to make a smooth transition between the floor and wall systems.

When using the top mount method of installation, you will cut the interior panel flush at the "bottom of floor" level on all sides.

When using the side mount method, you will cut the interior ICF panels running parallel with the floor beam to 2.5" above "bottom of floor" height. (ref. sec. 4.4)The interior panels at the end of the floor beams can be cut to "bottom of floor" height + panel height, or "bottom of floor" height + 2.5".

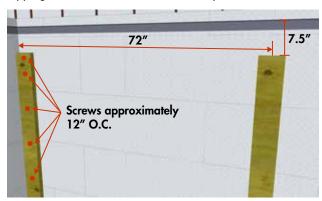
You may make a template of the beam cavity profile, measure where your beams will line up in the wall and cut the end of wall panels in advance to match the beam cavity profile or place floor panels and come back with an ICF saw to cut excess foam away from the beam cavity to match beam cavity profile.

For our example structure, we will use the side mount method. As you can see in the picture, the side wall and the end wall are cut to "bottom of floor" + 2.5".



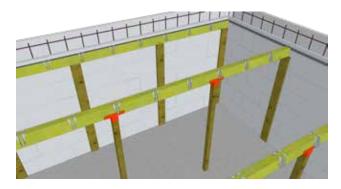
4.4 UTILITY ANGLE PLACEMENT

In order to provide a sheetrock attachment point (if necessary) around the perimeter of your intended ceiling (bottom of floor), install standard light gage 2"x2" utility angle at the marked "bottom of floor" height. Attach the utility angle to the BuildBlock webs 12" O.C. with #6 or #8 fine thread drywall screws (recommended), pan head screws as well as self tapping sheet metal screws are also acceptable.



4.5 VERTICAL GIRDER SUPPORTS PLACEMENT

The girders that will make up the main support for your shoring system will sit on a series of posts spaced on a 72" x 72" grid. The first step to erecting the shoring system is to install the vertical supports along your ICF walls. The first vertical support will be attached in the corner. The second support will be placed 72" on center from the ICF side wall (parallel to concrete beams being formed), and 72" O.C. thereafter. Your vertical supports and posts will be "bottom of floor" height -7.5" (girder height).

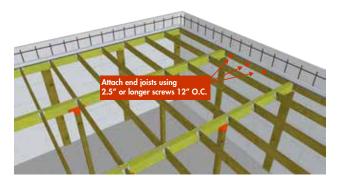


4.6 GIRDER PLACEMENT

Once vertical girder supports have been securely affixed to the ICF walls, install the first girder to the side wall.

Next, start to install the girders as the center posts are erected.

The girders should be connected to the posts by mechanical method such as the Ellis Manufacturing Co., purlin splicer.

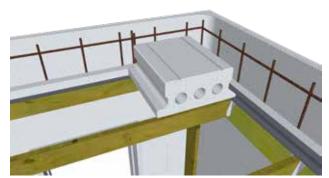


4.7 JOIST PLACEMENT

After girders with attached joist hangers are in place, drop the pre-cut joists into the available joist hangers. These do not need to be nailed in place, but care should be taken to ensure there is no gap more than 1/8" between joist ends and girders.

End joists are attached to the ICF webs with screws no shorter than 2.5'', 12'' O.C.

Place one screw high and one screw low at the ends of the joist, or stagger screws.



4.8 DECK PANEL PLACEMENT

Install Panels starting in one corner. If a cut panel is needed to fit a specific beam length, start with the male side facing out, and make the cut on the last panel, cutting excess from the female side. This will ensure that concrete does not flow into the hollow core.

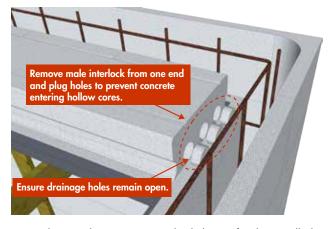
The steel ceiling attachment strip can be placed on either side of the panel so long as every row has an attachment strip where required.

Panels will interlock male to female in a row. If using a ceiling attachment strip, it may be easier to place one row at a time.

For smaller applications such as a small safe room, it may be easier to assemble rows of panels with glue on the ground and move them up to the shoring as a unit as the panels are very light weight.



As you place panels, take special care to ensure that the BuildDeck seams line up centered on the joists for optimum support.

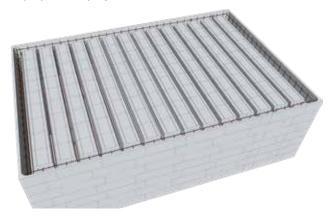


Depending on the mounting method chosen for the install, the male protrusions may be cut off the end panel and used to plug the opposite end to eliminate concrete waste caused by filling up hollow cores unintentionally.

If the panels are cut to the proper length there is no need to provide any attachment to the ICF walls. If the installer feels it is necessary, or if there are gaps or alignment issues, the panels may be attached to the ICF walls with foam to foam, low expansion glue. Make sure that drainage holes do not get filled.



Continue completing rows and sandwiching the steel attachment strips between the panels to complete the floor or roof deck. One crew member can be placing the bottom steel rebar in the bottom of the beam panels as you complete a row to prepare for tying steel.



Once your BuildDeck panels are placed and the bottom steel is set in the bottom of the beams it is time to place your steel.

Concrete codes require a minimum concrete coverage of .75" embedment around your rebar. If the project calls for a 2" concrete top cap, make sure your engineer has taken this into consideration.

For these applications a re-wire mesh will be required which may lower the live load rating as well as the span length and up lift strength.

4.9 REINFORCEMENT PLACEMENT



WARNING

ALL STEEL DESIGN MUST BE SPECIFIED BY THE ENGINEER OF RECORD.

When working with the engineer, ensure that your grid steel is specified on an increment of 6", 12" or 24". This will ensure easy placement of bottom beam steel.

Start by placing steel that runs parallel to the concrete beams. This steel must span up to or past the vertical steel bars in the ICF wall. This steel can sit on a number of commercially available rebar chairs. Steel wire chairs work best as they can be pressed into the BuildDeck panels for stability.

The bottom beam steel may be set in rebar chairs or hung

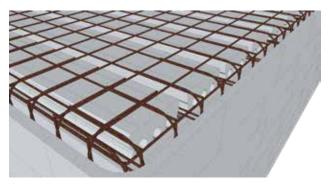
from the concrete cap grid steel with stirrups depending on the span, application and specifications of the Engineer. All steel must be tied at each end point and each intersection with code approved fastening procedures or mechanisms.

All steel splices must have an overlap of a minimum 40 diameters of the rebar. Bent angle rebar is required to connect grid steel and vertical steel in walls with ANSI approved bends, appropriate minimum overlap and code approved fastening procedures. Bottom beam steel may require bent angle rebar as per Engineered specifications.



When parallel concrete cap grid steel and bottom beam steel are properly and securely placed start to place perpendicular concrete grid steel. This steel must be appropriately tied, terminated and spliced as well.

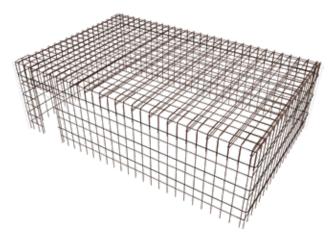
Place a straight edge across one corner and measure the distance from the bottom of the straight edge to the top of the highest level of grid steel. This measurement must be at least .75" to achieve minimum concrete embedment.



Place a 90 degree angle rebar at the end of each piece of concrete cap grid steel in each direction, tying in the concrete cap grid steel to the vertical wall steel.

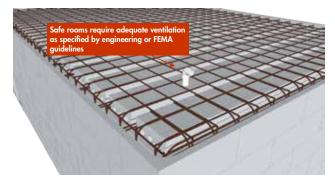
This is for example purposes only. The Engineer of record must specify final design.

Where additional support for ceiling elements is desired, the installer may drive 3"-3.5" screws through the steel attachment strip from the underside of the system which will allow for better adhesion between the concrete and the attachment strips prior to pouring.



The end result will be an interconnected steel reinforcing grid that can be designed by FEMA guidelines to be considered "near absolute protection" from natural disasters when combined with the strength of the monolithic concrete shell.

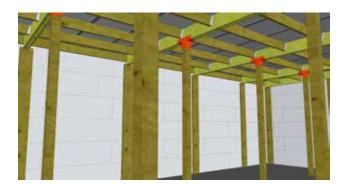
Follow all applicable codes regarding steel placement, tying and splicing.



4.10 UTILITIES PLACEMENT

Certain applications may require the passage of drainage and plumbing pipes, electrical conduit, vent and HVAC plenums or various other utility items to pass through levels. Additionally, safe rooms will require fresh air vents that will connect safe room space to attic space.

Work with your Engineers, designers and contractors to ensure that you know the proper placement of these items prior to pouring. Cut a hole the appropriate size for the intended item and place the item or a sleeve or block out for the item into the decking system. Seal all edges with foam to foam, low expansion glue. Extra shoring may be required around penetration openings.



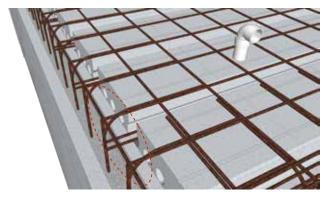
4.11 PRE-POUR CHECKLIST

Each installer should have a Pre-Pour checklist. This is a living document of sorts that may grow and change based on project needs and personal preferences. A few items that will always be vital to check prior to pouring are as follows:

- Check all ICF bracing for proper set up
- Inspect or install additional shoring where needed
- Go over utility diagrams one last time
- Inspect all deck shoring for stability
- Ensure that all ceiling attachment strips are in place
- Inspect all steel, steel splices and intersections for proper installation and concrete embedment
- Verify concrete mix ordered
- Place additional shoring in any areas of concern
- All finishing tools are present floats, shovels, vibrators, hand tools, trowels, etc.
- All accessories are present anchor bolts, extra lumber

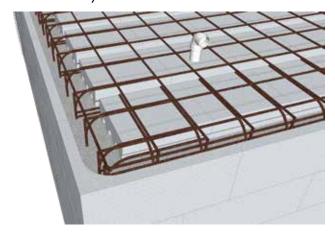
4.12 CONCRETE PLACEMENT

Pour ICF walls per installation guidelines outlined in the BuildBlock Installation Manual.



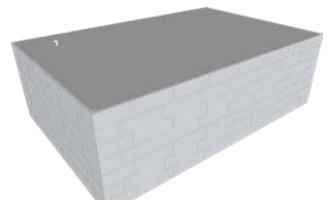
Take special care to keep edges clean and free of concrete where the concrete floor beams and the concrete wall meet. If concrete spills here and hardens, a cold joint can form at this critical point.

Fill the walls to a level just below where they intersect with the floor system. Do not place a cold joint at the intersection of the floor and wall systems.



Once the wall cavity is filled the appropriate level, start to fill the concrete beams. Vibrate the concrete very thoroughly as it is being placed. Monitor rebar to make sure that the minimum .75" concrete embedment is achieved on all sides of the steel.

Do not allow entrance by any person to the underside of the floor while concrete is being poured. Use caution beneath floor while floor is curing.



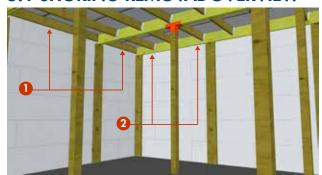
Continue to pour concrete, spreading and floating as you go. Fill to the top of floor height and finish using typical flatwork methods and procedures.

Place any anchor bolts in pre-marked locations as the floor is being finished, while concrete is still wet if applicable.

Do not use heavy equipment for finishing unless proper shoring precautions have been taken.

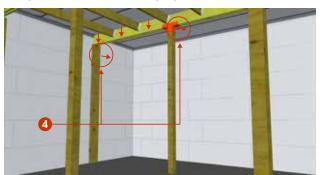
SECTION 5: BUILDDECK SHORING REMOVAL

5.1 SHORING REMOVAL OVERVIEW



Overview

- To remove shoring after the 28 day cure time has elapsed, start by removing screws from the joists that are attached to the walls between girders. Remove the joists and store for future projects.
- Remove screws in end wall vertical supports and end wall girder on one end.
- Carefully lower end wall vertical supports to a height that will allow removal of the first set of joists. Remove these joists and store for future projects.



- 4. Slide the first girder back by approximately 1.5", but let it rest on the tops of the side wall vertical supports still. This adjustment should allow the installer to slide one end of the joists in the next row over and safely remove the joists.
- 5. Once all joists in the section are removed, lower the girder, detach posts and store for future projects. Repeat this step until you reach the other end wall.
- 6. Remove remaining vertical supports and store for future projects.



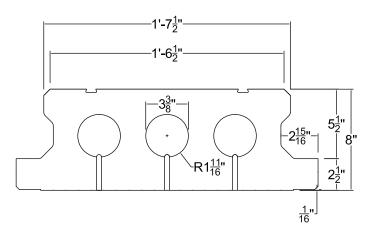
5.2 INTERIOR FINISH ATTACHMENT

The c-channel steel strips installed between each panel serve as attachment strips every 24" for sheet rock or other fire rated, code approved finish materials. If the finish application is a stucco or similar product that adheres directly to EPS, these attachment strips may not be required but are strongly recommended.

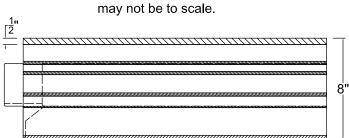




SECTION 6: BUILDDECK CAD DETAILS



Note: Due to variations in printers and print settings, this detail may or



Note:

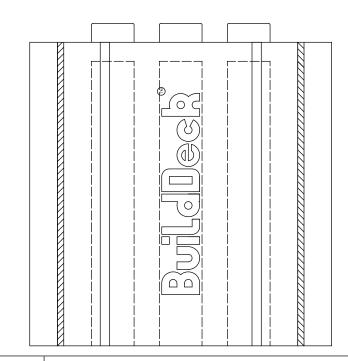
All concrete and steel design must be approved by the project specific Engineer of Record

Concrete Top Cap -Grid Steel Specf'd by Engineer of Record Optional 3.25" screw installed from below for additional reinforcement -BuildDeck BD-800 -Beam Steel Specf'd by Engineer of Record -Sheetrock #8 Course Thread Drywall Screw BB Supplied Steel Attachment Chanel

-24" NOMINAL-(BuildDeck Panel + Steel Attach. Strip)

-1'-117"--|1"|-

2-013-014-015-016-017-018-019-110-111-112-113-114-115-116-1





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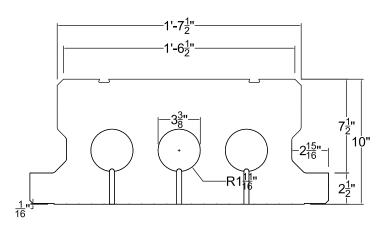
DATE/REV 11-17-09 NOTES BD-12+2

BUILDDECK 8"

CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.

SCALE NTS

DETAIL SHEET



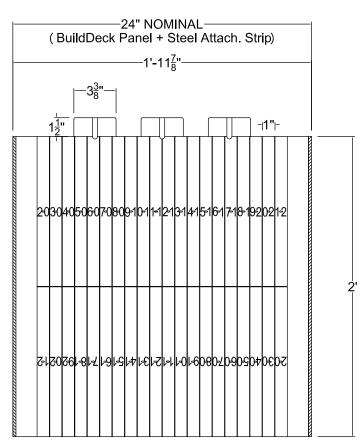
Note:

Due to variations in printers and print settings, this detail may or may not be to scale.



Note:

All concrete and steel design must be approved by the project specific Engineer of Record



Concrete Top Cap

Grid Steel Specf'd
by Engineer of Record

Optional 3.25" screw
installed from below for
additional reinforcement

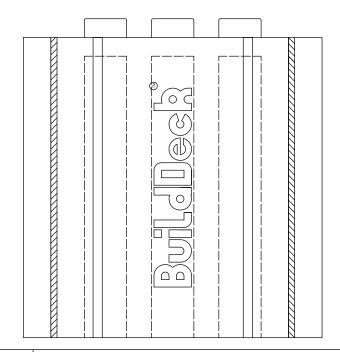
BuildDeck BD-1000

Beam Steel Specf'd
by Engineer of Record

Sheetrock

#8 Course Thread Drywall Screw

BB Supplied Steel Attachment Chanel





B Build B C S Y S T E M S

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NOTES BD-12+2

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SCALE NTS

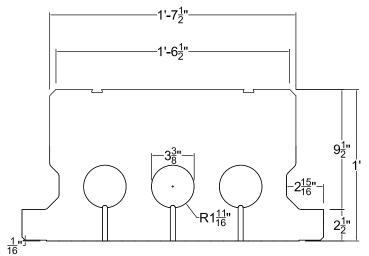
BUILDDECK 10"

11-17-09

DATE/REV

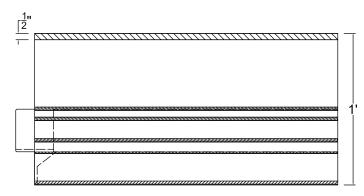
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DETAIL SHEET



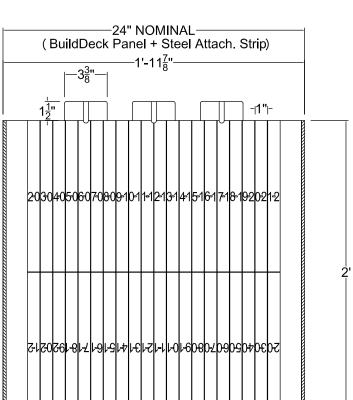
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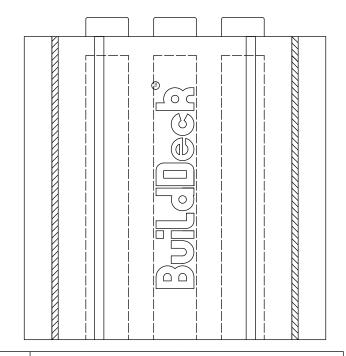


Note:

All concrete and steel design must be approved by the project specific Engineer of Record



Grid Steel Specf'd
by Engineer of Record
Optional 3.25" screw
installed from below for
additional reinforcement
BuildDeck BD-1200
Beam Steel Specf'd
by Engineer of Record
Sheetrock
#8 Course Thread Drywall Screw
BB Supplied Steel Attachment Chanel



SCALE NTS





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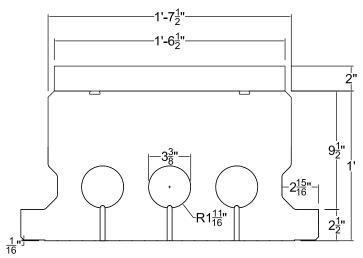
NOTES BD-12+2

CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.

BUILDDECK 12"

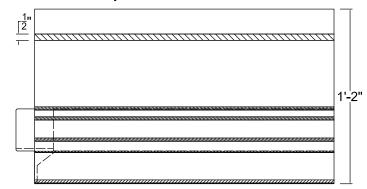
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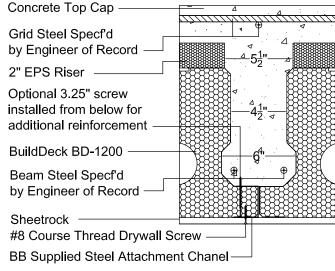
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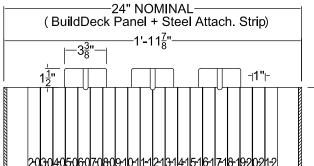
Due to variations in printers and print settings, this detail may or may not be to scale.

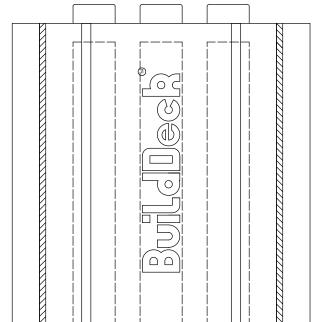


Note:

All concrete and steel design must be approved by the project specific Engineer of Record







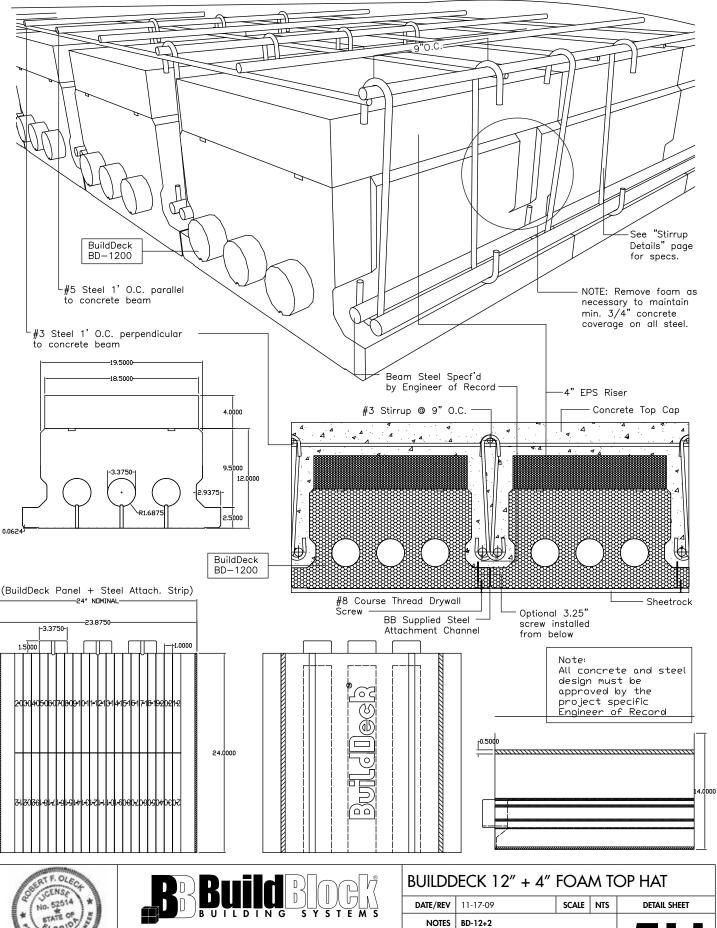




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BUILDDECK 12" + 2" FOAM TOP HAT

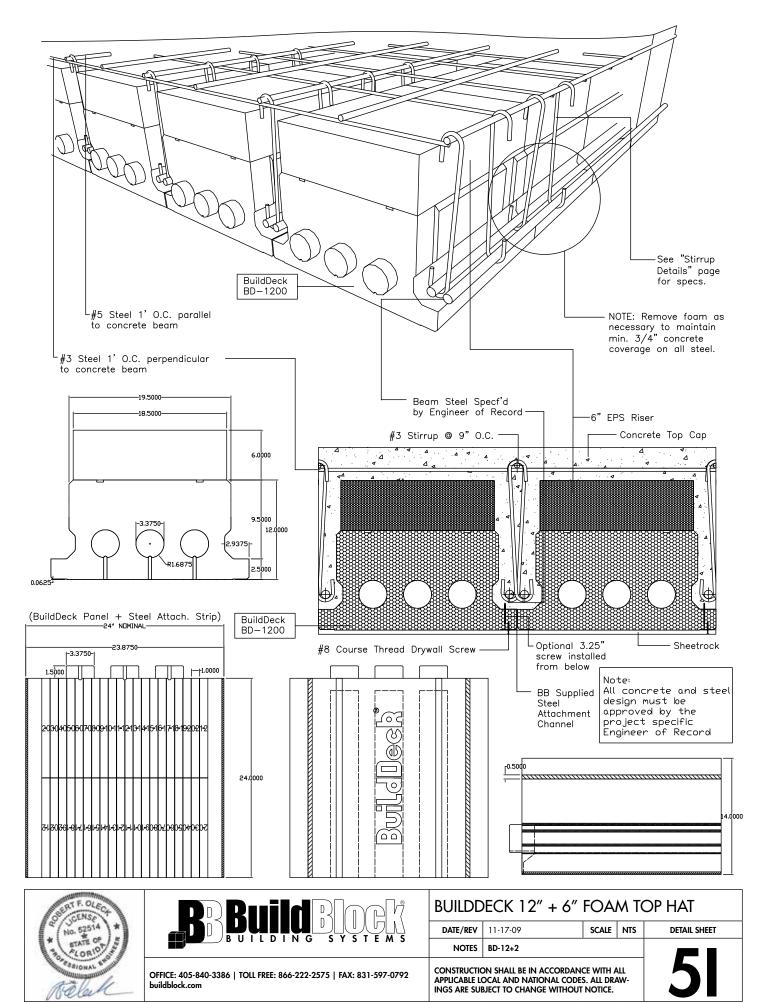
SCALE NTS DATE/REV 11-17-09 **DETAIL SHEET** BD-12+2 NOTES CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAW INGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.

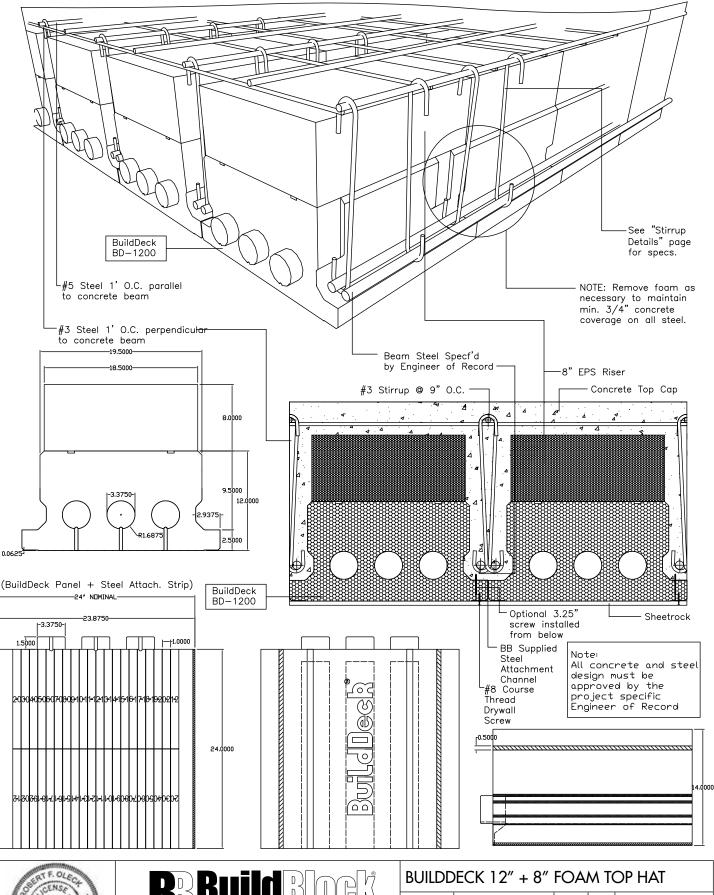




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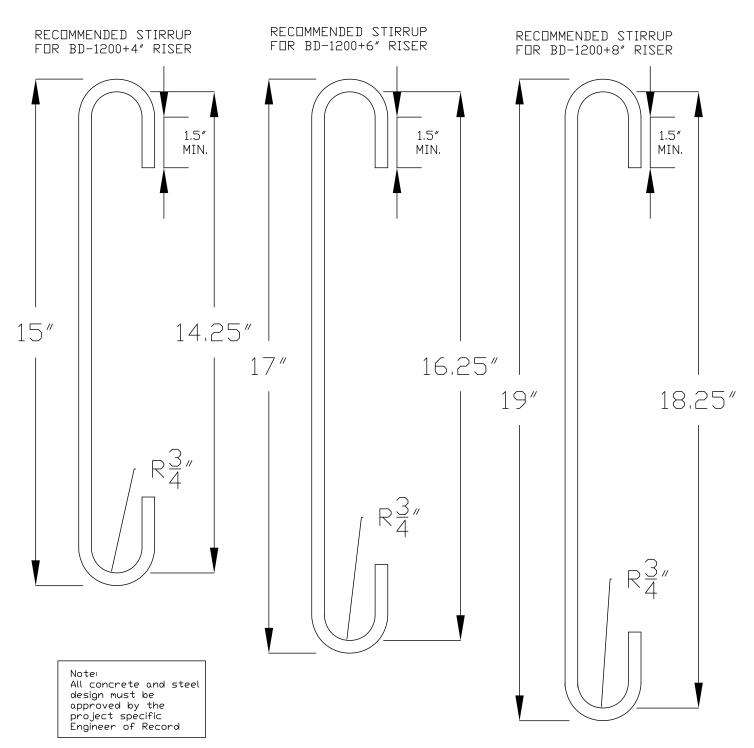






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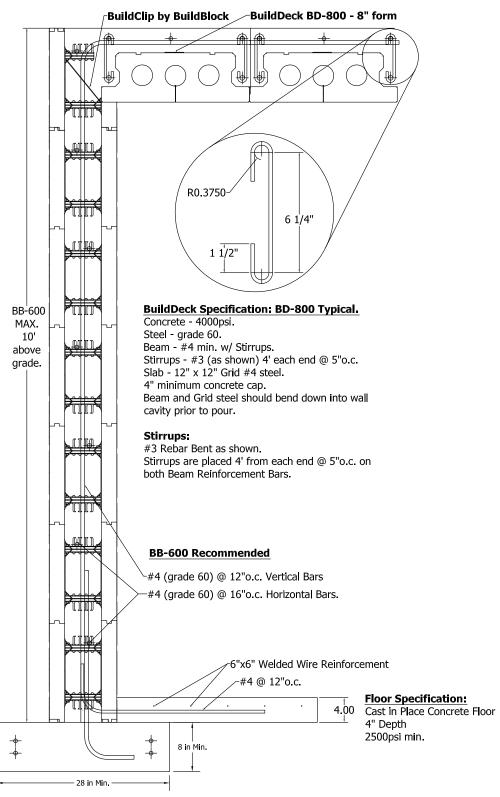


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BUILDDECK LONG SPAN STIRRUPS

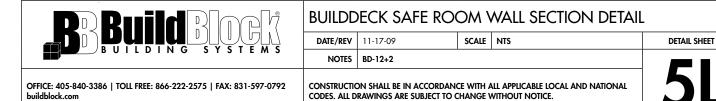


Footing Specifications:

28" wide min.

8" thick min.

Base of footing min 12" below grade. (IRC R403.1.3.2) Top of footing must be at frost line. (IRC R403.1.4.1)





BUILDDECK ROOF & FLOOR DECKING SYSTEM DESIGN, ENGINEERING, AND INSTALLATION MANUAL



SECTION 7: BUILDDECK ENGINEERING



September 22, 2009

bridge, highway & rail engineering entertainment engineering subaqueous investigation civil & site engineering structural design marine facilities geotechnics surveying

BuildBlock Building Systems, LLC 9701 N. Broadway Extension Oklahoma City, OK 73114

Attn: Mr. Justin Wallace

e: BuildBlock BuildDeck Shoring System

MEG File: 109735.00

Dear Mr. Wallace:

This letter is presented as McLaren Engineering Group's analysis of the proposed shoring system of the BuildDeck floor system. The BuildDeck floor system consists of 2' x 2' EPS panels with concrete poured on top and in between two adjacent panels, forming a web and a flange. The layout of the shoring system was proposed by BuildBlock and consists of the following: wooden 2x or 4x posts spaced at 6' on center in both directions that support 2x girders that support 2x joists spaced at 2' on center. Some girders may be double-ply. Blocking in between these plies may range from ½" thick plywood to 1-1/2" thick 2x material; whatever is needed to allow for the connection to the post. McLaren understands that there are three sizes of BuildDeck EPS panels that may be used; the panel that requires the most volume of concrete has been used for design.

The following are descriptions of the different abbreviations used in this letter.

SYP - Southern Yellow Pine

DFL (N) - Douglas Fir-Larch (North) or Douglas Fir-Larch

L.L. - Live Load

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JOIST TYPE	GIRDER TYPE	INTERIOR POST	EXTERIOR POST	ALLOWABLE L.L.
2X6 SYP #2	2X8 SYP #2	4X4 SYP STANDARD ² ³	2X6X10' SYP #21	50 psf
2X6 DFL (N) #2 ⁴	2X8 DFL (N) #24	4X4 DFL (N) #2 ^{2 3 4}	2X6X10' DFL (N) #24	32 psf ⁴
2X8 SYP #2	2X10 SYP #2	4X4 SYP STANDARD ² ³	4X4 SYP STANDARD ² ³	90 psf
2X8 DFL (N) #2	2X10 DFL (N) #2	4X4 DFL (N) #2 ^{2 3}	4X4 DFL (N) #2 ^{2 3}	79 psf

Table 1 - Allowable Loads

Table 1 Notes:

- a Note that post height doesn't include height of any jacking system employed.
- 1 2x6x10' post must be braced in the weak axis at mid-height. An acceptable bracing practice would be to screw the post to the ICF wall at 12" on center. If post height is 8' or less, acceptable post is as shown or 2x8x8' unbraced.
- 2 4X6 post may be substituted for 4x4 post.
- 3 Construction, standard, and #2 grades are interchangeable if same species is used.
- 4 This system that employs 2x6 DFL (N) #2 joists may only be used where construction type is explicitly light duty.

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M. G. McLAREN, P.C.

5728 Major Blvd, Suite 603 Orlando, FL 32819 Phone (407) 354-5411 Fax (407) 354-3466

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florida@mgmclaren.com
On the web: www.mgmclaren.com

Page 2 August 25, 2009

Table 2 summarizes the required Simpson (or other) connectors to be used.

MEMBERS	REQUIRED LOAD	CONNECTOR	AVAILABLE LOAD
2X6 JOIST TO 2X8 GIRDER	684 lb	LUS26 W/ (4) 10d NAILS	830 lb
2X8 JOIST TO 2X10 GIRDER	930 lb	LUS28 W/ (6) 10d NAILS	1055 lb
BEAM TO 4X COLUMN	STABILITY	(2) LPC4Z¹ OR ELLIS CAP	STABILITY
BEAM TO 2X6 COLUMN	STABILITY	LPC6Z ¹	STABILITY
GIRDER TO 2X4 BLOCKS	STABILITY	(4) 16d NAILS	STABILITY

Table 2 - Required Connectors

Table 2 Notes:

- a Values have been taken from the 2009-2010 Simpson Wood Construction Connectors catalog.
- 1 Use (8) 10d nails into the beam and (8) 10d nails into the post.

The following are recommendations that must be followed to ensure that the shoring system functions properly:

- 1) The shoring system must be somehow braced laterally in the direction parallel to the joist span. The system is considered braced if exterior posts are screwed to an ICF wall.
- 2) All instructions and precautions for Simpson connectors and Ellis screw jacks and post caps must be followed.
- 3) All joists must be snug fit against the joist hangar on all four sides of contact.

Included below are pages 11, 19 and 20 from SEI/ASCE37-02 which is a standard for the design loads on structures during construction. Page 11 includes definitions and the table on page 19 classifies construction into four different types: very light duty, light duty, medium duty, and heavy duty. Each type has an associated uniform load which does not include dead load, construction dead load, or fixed material loads, so this uniform load can be considered a live load in the BuildDeck application. There are descriptions of these four types of construction to the right of the table and onto page 20. Two characteristics of heavy duty construction are concrete transport and placement using motorized buggies and material storage.

Note that in no case should heavy duty construction take place on the proposed shoring system; however it is acceptable to store limited quantities of 20 gage sheet metal and EPS panels over the girders. Any storage over the joists should be kept very minimal at all times.

It is the opinion of McLaren that the main construction type for the BuildDeck application is medium duty and therefore most systems require the use of a live load of 50 psf. Therefore, the shoring framing system employing 2x6 DFL (N) #2 joists should not be used unless construction type is explicitly light duty, which may be the case in some shoring applications.



Page 3 August 25, 2009

SEI/ASCE 37-02

COMMENTARY

C4.0 CONSTRUCTION LOADS

C4.1 General Requirements

The loads for some temporary structures, such as those that retain lateral pressures of earth, are not defined in Section 4; refer to Section 5 for lateral pressures of earth

Standards and other documents applicable to specific materials or methods of construction have been developed and are recognized and used extensively.

STANDARD

4.0 CONSTRUCTION LOADS

4.1 General Requirements

The provisions of this section shall be used to define the construction loads for the design of both temporary structures and permanent structures subject to loads during construction. These loads are to be combined with other applicable loads per the requirements of Section 2.

When a construction loading is covered in another document that is acceptable to the authority having jurisdiction and written to address a specific material or method of construction, the more applicable document shall be permitted to be followed.

Stairs, ladders, and elevators are not addressed in this standard.

4.1.1 Definitions

Construction loads: those loads imposed on a partially completed or temporary structure during and as a result of the construction process. Construction loads include, but are not limited to, materials, personnel, and equipment imposed on the temporary or permanent structure during the construction process.

Construction dead load, C_D : the dead load of temporary structures that are in place at the stage of construction being considered. The dead load of the permanent structure, either partially complete or complete, is not included in C_D ; the dead load of the permanent structure is defined as *dead load*, D, in Section 3.1.

Individual personnel load: a concentrated load of 250 lb (1.1 kN) that includes the weight of one person plus equipment carried by the person or equipment that can be readily picked up by a single person without assistance.

Working surfaces: floors, decks, or platforms of temporary or partially completed structures which are or are expected to be subjected to construction loads during construction.

4.2. Material Loads

The material dead loads consist of two categories:

- 1. fixed material loads (FML)
- 2. variable material loads (VML)

The FML is the load from materials that is fixed in magnitude. The VML is the load from materials that varies in magnitude during the construction process. If the local magnitude of a material load varies during the

C4.2. Material Loads

This section separates material dead loads into two categories: FML and VML, which are separated to permit the use of an appropriate load factor for each category in strength design. This approach recognizes the difference in the variability of the load between the two categories.

This section addresses the loads from materials and is not intended to apply to equipment loads. Per-

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Page 4 August 25, 2009

SEI / ASCE 37-02

STANDARD

4.8 Application of Loads

4.8.1 Combined Loads

The design construction load shall include the critical combination of personnel, equipment, and material loads.

4.8.1.1 Working Surfaces. Structures supporting working surfaces as defined in Section 4.1 shall be designed for the combined material, personnel, equipment, and other applicable construction loads.

When the construction operation fits the definition in Table 2, the designer is permitted to design for the tabulated uniform loads as the vertical load from the combination of personnel, equipment, and material in transit or staging. When the construction operation does not fit the definitions in Table 2, the design shall be for the actual loads. Concentrated loads shall be considered separately.

Table 2 Classes of Working Surfaces for Combined Uniformly Distributed Loads

Operational Class	Uniform Load psf (kN/m²)
Very light duty: sparsely populated with personnel; hand tools;	20 (0.96)
very small amounts of construction materials	
Light duty: sparsely populated	25 (1.20)
with personnel; hand operated equipment; staging of materials	
for lightweight construction Medium duty: concentrations of personnel;	50 (2.40)
staging of materials	50 (2.10)
for average construction	
Heavy duty: material placement	75 (3.59)
by motorized buggies;	
staging of materials for	
heavy construction	

 $^{^{\}rm a}$ Loads do not include dead load, D; construction dead load, $C_{\rm D};$ or fixed material loads, $C_{\rm FML}.$

SEI/ASCE 37-02

COMMENTARY

C4.8 Application of Loads

Construction loads depend very much on the specific planning and processes of construction. This section includes rules for applying and combining the various loads, as well as traditional minimums for several common construction processes.

C4.8.1 Combined Loads

The combination of the various forms of construction loads, materials, personnel, and equipment is an important step in engineering for construction, requiring careful application of professional judgment.

C4.8.1.1 Working Surfaces. It is traditional to design many working surfaces for a uniformly distributed load that is meant to include all construction loads, except for materials in their final position.

Temporary structures have often been designed, advertised, and specified by the light, medium, and heavy duty ratings given in Table 2. This standard also applies to partially completed structures, and the same terminology is adopted. Different styles of construction and different segments of the construction industry have different traditions for design loads on partially completed structures during construction, and this section of the standard is an attempt to unify the industry on a common basis.

Examples of construction operations that have traditionally been designed for the loads given in the table are

Very light duty:

Roofing, reroofing, excepting situations with stockpiles of ballast

Access catwalks

Painting, caulking

Maintenance using hand tools

Light duty:

Light frame construction

Concrete transport and placement by hose and concrete finishing with hand tools

Medium duty:

Concrete transport and placement by buckets, chutes, or handcarts

Concrete finishing using motorized screeds

Masonry construction with tile or hollow lightweight concrete units

Structural steel erection or concrete reinforcing steel placement

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Page 5 August 25, 2009

DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION

STANDARD

COMMENTARY

Heavy duty:

Concrete transport and placement using motorized buggies

Masonry of brick or heavy-weight concrete units Material storage

Conflicts between provisions of this section and those in ASCE 3-91 and ASCE 9-91 are acknowledged.

Following are examples of working surfaces that do not fall under Table 2:

Roofs for which design is controlled by building code live load or snow loads that are less than values in Table 2.

Attics or hung ceilings that provide access for maintenance, installation of utilities, and emergency services such as firefighters.

These working surfaces must be addressed in accordance with Sections 4.8.1.1 and 4.8.4.

C4.8.1.2 Specification of Temporary Structures. This requirement will encourage uniformity in terminology for capacity of scaffolds and similar structures.

4.8.2 Partial Loading

loads shall be as given in Table 2.

The full intensity of the construction load applied only to a portion of the length of a structure or member shall be considered if it produces a more unfavorable effect than the same intensity applied over the full length of the structure or member.

4.8.1.2 Specification of Temporary Structures. When

temporary structures are specified by load name, the

names of the load class and the magnitude of design

4.8.3 Reduction in Construction Loads

4.8.3.1 Material Loads. No reduction is allowed for fixed or variable material loads, except to the extent that small amounts of material in transit or staging are included in uniformly distributed personnel, equipment, and material loads, such as those in Table 2

4.8.3.2 Personnel and Equipment Loads. When justified by an analysis of the construction operations, members having an influence area of 400 ft2 (37.16 m²) or more may be designed for a reduced uniformly distributed personnel and equipment load determined by applying the following formula:

$$C_P = L_o (0.25 + 15/\sqrt{A_I})$$
 (4-6)

$$C_{PSI} = L_o (0.25 + 4.57/\sqrt{A_I})$$
 (4-6 SI)

(4-6 SI)

C4.8.2 Partial Loading

Partial-length loads on a beam or truss may produce higher shear on a portion of the span than a fulllength load. Checkerboard loadings on floors and multistory frames produce the highest positive and negative moments. Cantilevers cannot rely on a possible construction load on the anchor span for equilibrium. ASCE 7-95 describes other possible conditions of designing members or floors for partial loading.

C4.8.3.2 Personnel and Equipment Loads. Uniformly distributed loads are a convenient substitute for computing the combined effect of several concentrated loads. As such they are generally calibrated to a particular area. For smaller areas, the concentrated loads control structural design. The nature of transient concentrated loads, such as personnel and equipment, is that their spacing is not uniform, thus, for areas larger than the calibration area, the uniform load may be unnecessarily conservative.



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Page 6 August 25, 2009

If you have any questions and or comments please contact me at (407) 354-5411 or via email at roleck@mgmclaren.com.

Very truly yours The Office of McLaren Engineering Group M.G. Mclaren, P.C.

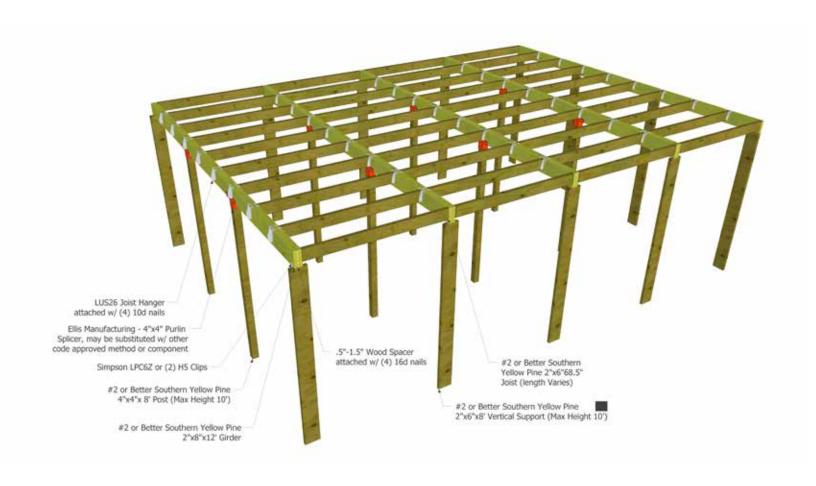


Robert F. Oleck, Ph.D., P.E. Regional Director

cc: MGM, WRM, DWH,

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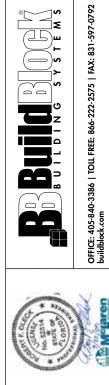
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Span Le	Span Length (Ft.)	8	6	10	11	12	13	13 14	15	16	17	18	19	20
Qty. Rein. Steel Rqd.	Bar Size Designation						Liv	Live Load	pε					
2	#4	357	271	197	145	108	08	89	42	29	19	10	3	×
2	9#	443	312	226	166	123	6	89	49	32	24	14	9	×
2	9#	516	360	259	190	141	105	62	89	42	29	19	10	4
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	Steel Yield Strength	ngth		F'y = 60 ksi) ksi									
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	Long Term Deflection	ection		Def < L/480	/480									
	Stirrup Reinforcement in Beam	ement ir	า Beam	#3 rebal	#3 rebar, 4 ft each end @ 5" O.C.	ch end @	95" O.C.							
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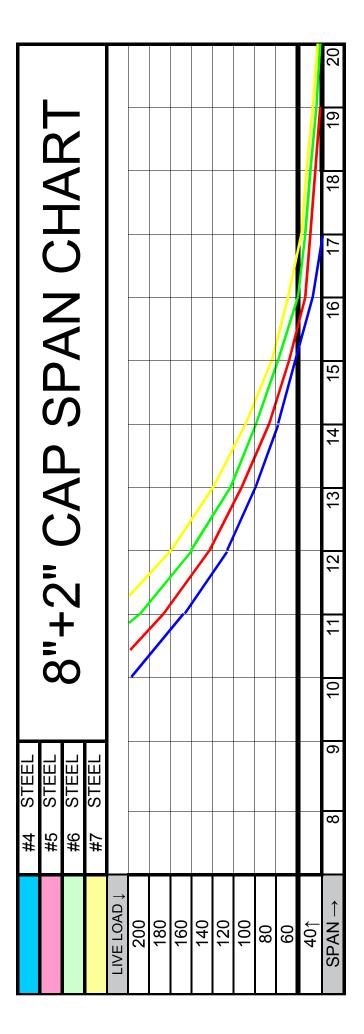
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BUILDDECK 8" +2" CONCRETE CAP TABLE

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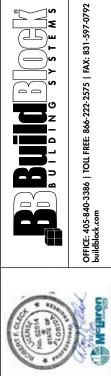




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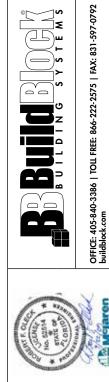
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BuildDeck - 8" Deck with 3" Concrete Cap

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2	#	414	315	244	192	152	117	88	92	48	33	22	13	2	×	×
2	42	618	437	318	236	178	135	103	78	28	41	29	18	10	က	×
2	9#	730	511	369	274	206	156	119	91	89	51	37	25	16	∞	×
2	2 #	849	591	425	314	236	179	137	106	81	09	45	32	22	14	9
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	Steel Yield Strength	ngth		F.y = 60	KSI											
	Addtl. Applied Dead Ld. Incl.)ead Ld.		DL = 15 psf	bst											
	Long Term Deflection	ection		Def < L/480	480											
	Stirrup Reinforcement in Beam	ement ir	ו Beam	#3 rebar	#3 rebar, 4 ft each end @ 5" O.C.	h end @	5" O.C.									
	Slab Reinforcement	nent		12" × 12	12" x 12" Grid #4 Stee	Steel										

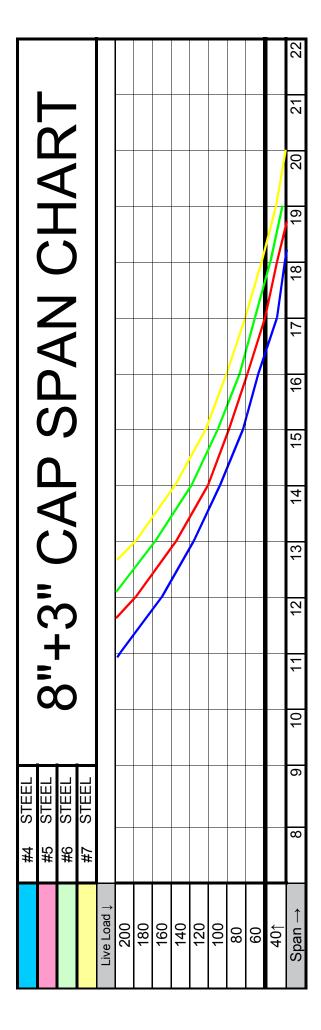
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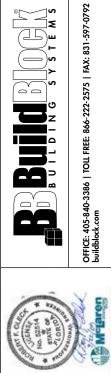




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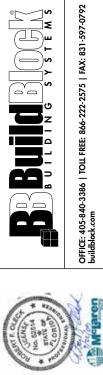
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2	‡	469	357	276	216	171	136	108	98	29	51	39	28	19	10	2	×	×
2	£	752	280	144	331	727	194	150	117	06	69	25	38	56	16	8	1	×
2	9#	866	203	512	383	167	223	174	136	106	82	63	47	34	23	14	2	1
2	2 #	1165	815	069	440	333	257	199	156	122	96	74	25	43	31	22	13	9
Assumptions:	Concrete Design Yield Strength	n Yield §	Strength	F'c = 4000 psi	00 psi													
	Steel Yield Strength	ngth		F'y = 60 ksi	ksi													
	Addtl. Applied Dead Ld. Incl.	Dead Ld.	Incl.	DL = 15 psf	psf													
	Long Term Deflection	ection		Def < L/480	480													
	Stirrup Reinforcement in Beam	ement ir		#3 rebar, 4 ft each end @ 5" O.C.	, 4 ft eac	th end @	35" O.C.											
	Slab Reinforcement	nent		12" x 12	12" x 12" Grid #4 Steel	Steel												
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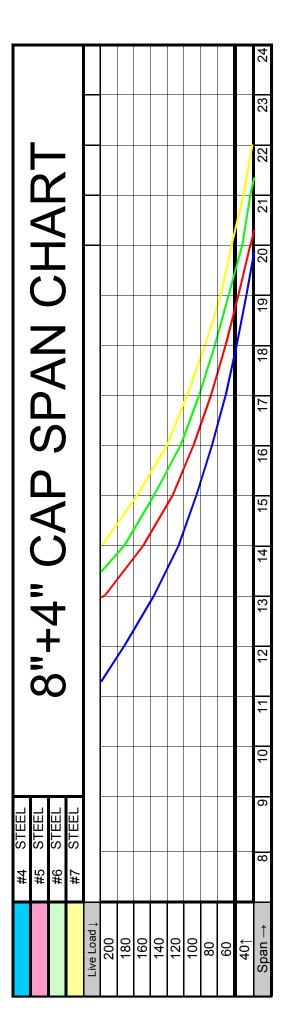




BUILDDECK 8" +4" CONCRETE CAP TABLE

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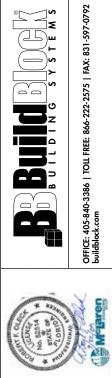


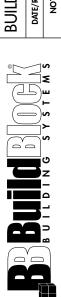
BUILDDECK 8" +4" CONCRETE CAP CHART

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			_	BuildDecl	il De	ck -	In Deck With 2" Concrete Cap	Dec	ik W	√ith	2" (Son	crei	te C	àp						
Span Le	Span Length (Ft.)	8	6	10	11	12	13	14	15 16	16	17	17 18 19	19	20	21	22 23		24	25	26	27
Qty. Rein. Steel Rqd.	Bar Size Designation										Live Load	yad									
2	#	479	368	288	230	201	149	122	66		26	44	32	22	14	7	×	×	×	×	×
2	42	753	280	424	318	244	188	147	115	68	89	52	39	28	19	11	4	×	×	×	×
2	9#	626	989	499	372	283	217	169	132	104	81	62	47	32	25	16	6	3	×	×	×
2	2 #	1151	803	280	431	326	251	195	153	120	94	74	25	44	32	23	15	8	2	×	×
Assumptions:	Assumptions: Concrete Design Yield Strength F'c = 4000 psi Steel Yield Strength F'y = 60 ksi Addti. Applied Dead Ld. Incl. DL = 15 psf Long Term Deflection Def < L/480 Stirrup Reinforcement in Beam #3 rebar, 4 ft each er Slab Reinforcement 12" x 12" Grid #4 Ste	n Yield S ngth Dead Ld. ection ement in nent	strength Incl. Beam	F'c = 4000 psi F'y = 60 ksi DL = 15 psf Def < L/480 #3 rebar, 4 ft e 12" x 12" Grid	Fc = 4000 psi Fy = 60 ksi DL = 15 psf Def < L/480 #3 rebar, 4 ft each end 12" x 12" Grid #4 Steel	Fc = 4000 psi Fy = 60 ksi DL = 15 psf Def < L/480 #3 rebar, 4 ft each end @ 5" O.C. 12" x 12" Grid #4 Steel	nd @ 5" O.C. bel Designs Mist be reviewed and approved by the project specific engineer of record	ריים מי	EVIEW	CHA CH	NAPRO C	VED 83	THE P		S B B B B B B B B B B B B B B B B B B B			. DE REC			
אור היויי	ייס ייס		,	100		ן יי		2		֚֚֚֚֚֚֚֚֚֚֚֚֚֡֝֝֝֝֝֝֝֝֝֝֝֝֝֓֓֓֓֝֝֡֓֓֝֝֡		ָ ֡ נ	-	2001	2	; 2		֭֡֝֝֝֝֟֝֝֝֝֟֝֝֝֝֟֝֝֝֝֜֜֟֝֝֝֜֜֜֟֝֝֜֜֜֝֝֡֜֜֝֝֡			

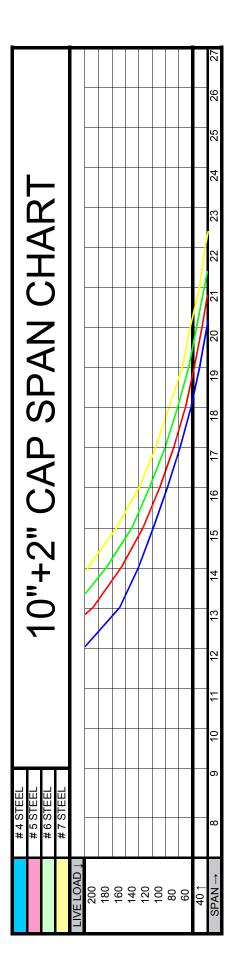


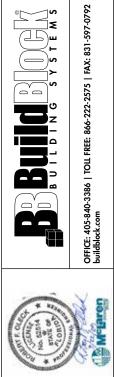


BUILDDECK 10" +2" CONCRETE CAP TABLE

NOIES			
CONSTRUCTION SHALL BE IN ACCORDANC SUBJECT TO CHANGE WITHOUT NOTICE.	CE WITH A	CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.	•

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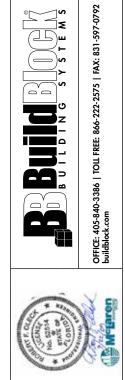


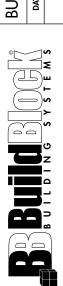
BUILDDECK 10" +2" CONCRETE CAP CHART

			NOTES
DETAIL SHEET	NTS	SCALE	DATE/REV

CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.

				Buil	dDe	BuildDeck - 10" Deck With 3" Concrete Cap	10"	Dec	× ×	Vith	3" (Con	cre	te C	ap						
Span Le	Span Length (Ft.)	8	6	10	11	12	13 14	14	15	16	17	18	19	20	21	22	23	24	25	26	27
Qty. Rein. Steel Rqd.	Bar Size Designation									٦	Live Load	bad									
2	#	236	411	321	254	201	165	134	109	87	71	99	43	34	23	14	9	×	×	×	×
2	\$#	820	629	522	412	316	245	192	151	119	94	74	99	42	30	20	13	2	×	×	×
2	9#	1214	894	029	486	370	286	224	177	140	111	88	69	53	40	29	19	12	2	×	×
2	2 #	1510	1055	292	899	432	334	261	206	164	130	103	82	65	20	38	28	18	11	8	×
Assumptions:	Assumptions: Concrete Design Yield Strength	n Yield S		F'c = 4000 psi	00 psi																
	Steel Yield Strength	ngth		F'y = 60 ksi	ksi																
	Addtl. Applied Dead Ld. Incl.	ead Ld.	Incl.	DL = 15 psf	psf																
	Long Term Deflection	ection		Def < L/480	480																
	Stirrup Reinforcement in Beam	ement in	Beam	#3 rebar	., 4 ft eac	#3 rebar, 4 ft each end @ 5" O.C.	5" O.C.														
	Slab Reinforcement	nent		12" x 12	12" x 12" Grid #4 Steel	Steel															
ALL TABLES	ALL TABLES PROVIDED FOR ESTIMATION PURPOSES ONLY. ALL DESIGNS MUST BE REVIEWED AND APPROVED BY THE PROJECT SPECIFIC ENGINEER OF RECORD	ESTIMA	ATION PU	RPOSE	S ONLY.	ALL DE	SIGNS M	UST BE	REVIEW	VED AN	D APPR	OVED B	Y THE P	ROJEC.	T SPECI	FIC ENC	SINEER	OF REC	ORD.		





BUILDDECK 10" +3" CONCRETE CAP TABLE

DATE/REV	SCALE NTS	NTS	
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CONSTRUCTION SHALL BE IN ACCORDANG SUBJECT TO CHANGE WITHOUT NOTICE.	ICE WITH A	CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.	

DETAIL SHEET

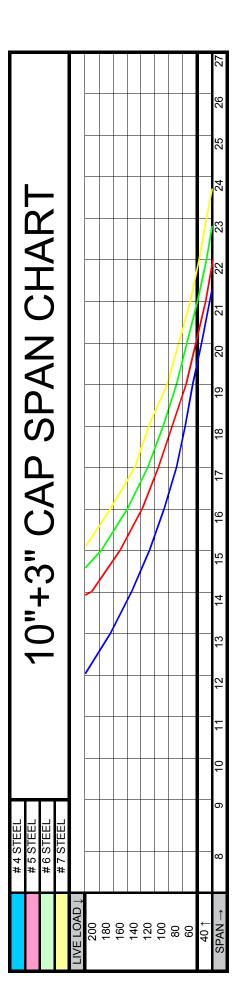






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DATE/REV	SCALE	NTS	DETA
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TAIL SHEET

				BuildDeck-	dDe	CK-	10"	10" Deck With 4" Concrete Cap	ck V	Vith	4"	Con	cre	te C	ap						
Span Le	Span Length (Ft.)	8	6	10	10 11	12		13 14	15	16	17	18	19	20 21	21	22 23	23	24	25	26	27
Qty. Rein. Steel Rqd.	Bar Size Designation										Live Load	1									
2	#4	265	453	354	279	224	179	145	117	94	22	29	46	34	25	16	6	2	×	×	×
2	42	942	732	629	466	380	313	250	199	159	127	101	62	62	47	35	24	15	7	×	×
2	9#	1358	1058	830	622	476	371	292	232	185	149	119	92	22	26	45	33	24	15	8	×
2	2 #	1821	1349	626	730	222	433	341	270	216	175	140	113	91	72	25	44	33	23	15	8
Assumptions:	Concrete Design Yield Strength	n Yield S	trength	F'c = 4000 psi	isd 000																
	Steel Yield Strength	ngth		F'y = 60 ksi) ksi																
	Addtl. Applied Dead Ld. Incl.	Dead Ld.	lucl.	DL = 15 psf	, psf																
	Long Term Deflection	ection		Def < L/480	/480																
	Stirrup Reinforcement in Beam	ement in	Beam	#3 reba	r, 4 ft eaα	#3 rebar, 4 ft each end @	95" O.C.														
	Slab Reinforcement	nent		12" × 12	12" x 12" Grid #4 Steel	t Steel															
ALL TABLES	ALL TABLES PROVIDED FOR ESTIMATION PURPOSES ONLY. ALL DESIGNS MUST BE REVIEWED AND APPROVED BY THE PROJECT SPECIFIC ENGINEER OF RECORD	ESTIM	TION PL	JRPOSE	S ONLY.	ALL DE	SIGNS N	IUST BE	REVIEV	VED AN	D APPR	OVED B	Y THE F	ROJEC	T SPECI	FIC ENG	INEER	OF REC	ORD.		

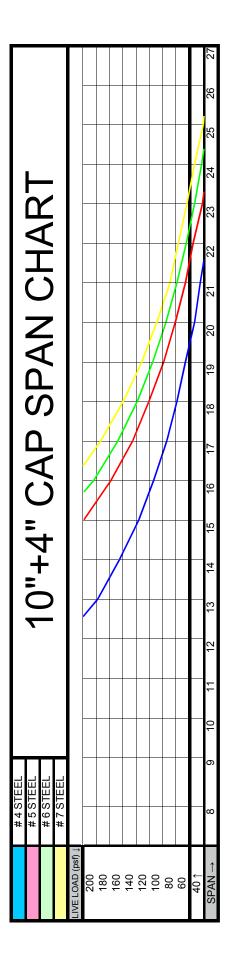




BUILDDECK 10" +4" CONCRETE CAP TABLE

NOTES				•
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DETAIL SHEET







BUILDDECK 10" +4" CONCRETE CAP CHART

			NOTES	
DET	NTS	SCALE	DATE/REV	

TAIL SHEET

							CK-	12		eck	X	ith	BuildDeck - 12" Deck With 2" Concrete	Cor	ıcre	ete	S Cap	d							
Span Le	Span Length (Ft.)	8		10	9 10 11 12	12		14	15	13 14 15 16 17	17	18	19	20	20 21 22	22	23 24	24	25	56	27 28		29	30	31
Qty. Rein. Steel Rqd.	Bar Size Designation												Live Load	bad											
2	7 #	602	464 366 292	366	292	237	193	159	131	108	06	74	61	49	40	31	23	16	6	×	×	×	×	×	×
2	5 #	946	982	585	585 474	390	316	251	200	161	130	105	83	29	52	40	30	21 1	13	7	×	×	×	×	×
2	9#	1342	1049	824	617	473	698	291	232	187	151	121	86	62	62	49	38	28 2	20 1	12	9	×	×	×	×
2	L #	1780	1344 975	975	727	554	431	339	270	217	175	142	115	93	74	26	46	36 2	26 1	18	11	2	×	×	×
Assumptions:	Assumptions: Concrete Design Yield Strength F'c = 4000 psi	n Yield	Strength	F'c = 4	4000 ps	_																			
	Cton Viola Ctronath	4		100 100																					

Steel Yield Strength F'y = 60 ksi
Addtl. Applied Dead Ld. Incl. DL = 15 psf
Long Term Deflection Def < L/480
Stirrup Reinforcement in Beam #3 rebar, 4 ft each end @ 5" O.C.
Slab Reinforcement 12" x 12" Grid #4 Steel
ALL TABLES PROVIDED FOR ESTIMATION PURPOSES ONLY. ALL DESIGNS MUST BE REVIEWED AND APPROVED BY THE PROJECT SPECIFIC ENGINEER OF RECORD.



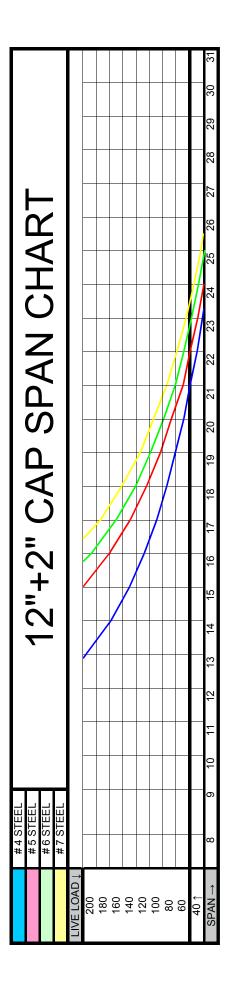
DATE/REV	SCALE	NTS
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DETAIL SHEET		
SCALE NTS		
SCALE		
DATE/REV	NOTES	

BuildDeck - 12" Deck With 3" Concrete Cap

									-					_			•	_	-	_	_	-		
Span Length (Ft.)		∞	တ	10	7	12	13	14	15	16	17	9	19	20	7	22	23 2	24 2	25 2	26 27	7 28	3 29	30	31
Bar Size Designation												Ë	Live Lo	Load										
#		629	202	399	318	256	509	171	140	115	94	77	62	20	39	29 2	22 14	8	×	×	×	×	×	×
9#		1043	810	644	521	427	354	596	249	201	163	132	107	98	89	54 4	41 31	1 21	1 13	9 9	×	×	×	X
9#		1492	1165	931	757	282	459	364	291	235	190	155	126	102	82 (99	52 40	29	9 21	13	7	×	×	X
2 #		1992	1560	1214	206	693	-	427	341	275	223	182	149	121	8 66	9 08	64 50	39	30	20	14	2	×	X
Accommendation Concrete Decide Viold Strength Fig 4000 and	5.	Viold C	dt-cond.	L'- 11	100																			

Assumptions:

Concrete Design Yield Strength F'c = 4000 psi Steel Yield Strength F'y = 60 ksi DL = 15 psf Def < L/480 Addtl. Applied Dead Ld. Incl.

Long Term Deflection

Stirrup Reinforcement in Beam #3 rebar, 4 ft each end @ 5" O.C.
Slab Reinforcement 12" x 12" Grid #4 Steel
ALL TABLES PROVIDED FOR ESTIMATION PURPOSES ONLY. ALL DESIGNS MUST BE REVIEWED AND APPROVED BY THE PROJECT SPECIFIC ENGINEER OF RECORD.

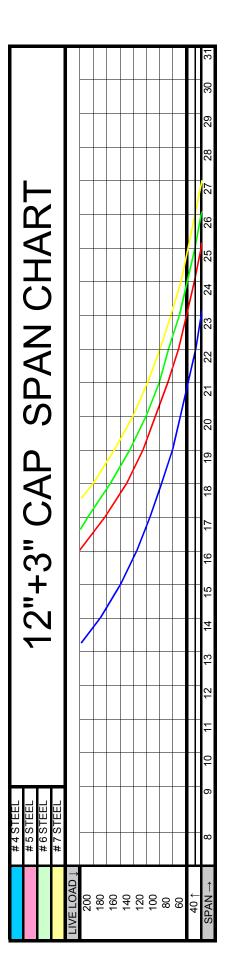


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BUILDDECK 12" +3" CONCRETE CAP CHART

DATE/REV SCALE NTS DETAIL SHEET

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CONSTRUCTIC SUBJECT TO CI	CONSTRUCTION SHALL BE IN ACCORDANC SUBJECT TO CHANGE WITHOUT NOTICE.	JE WITH A	CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES, ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.	

BuildDeck - 12" Deck With 4" Concrete Cap

-1							_
	31		×	×	×	2	
	30		×	×	×	8	
	29		×	×	7	15	
	28		×	7	15	24	
	27		×	14	22	32	
	26		×	22	32	43	
	25		2	32	42	54	
•	24		11	43	22	89	
	23		19	26	20	85	
	22		28	1.4	98	104	
	21		38	88	106	127	
	20	oad.	09	110	130	154	
	19	Live Load	89	134	158	181	
	18	7	80	163	193	228	
	17		66	193	236	278	
	16		121	227	290	341	
	15		149	269	358	421	
	14	12 13 14	182	320	446	525	
	13		224	384	299	663	
	12		343 276	494	684	098	
	11		343	999	829	1378 1110	טטט אפ
	9 10		431	104	1019	1378	F'c = 4
			549	883	1277	1720	trendth
	8		715	1139	1637	2197	2 NaiV
	Span Length (Ft.)	Bar Size Designation	#4	9#	9#	2#	Assumptions: Concrete Design Yield Strength Fig = 4000 psi
	Span Le	Qty. Rein. Steel Rqd.	2	2	2	2	Assumptions.

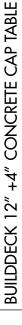
Concrete Design Yield Strength Steel Yield Strength

F'y = 60 ksiDL = 15 psf

Addtl. Applied Dead Ld. Incl.

Long Term Deflection Def < L/480
Stirrup Reinforcement in Beam #3 rebar, 4 ft each end @ 5" O.C. Slab Reinforcement 12" x 12" Grid #4 Steel

ALL TABLES PROVIDED FOR ESTIMATION PURPOSES ONLY. ALL DESIGNS MUST BE REVIEWED AND APPROVED BY THE PROJECT SPECIFIC ENGINEER OF RECORD.

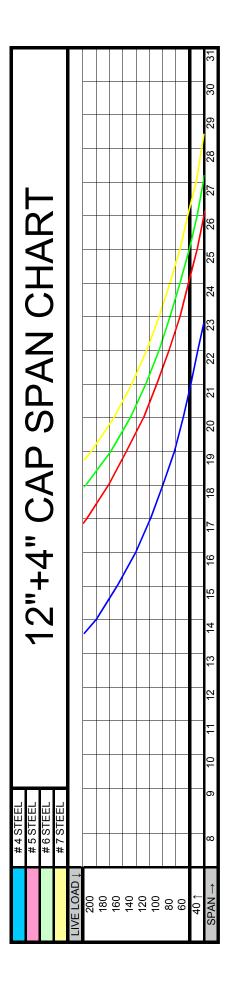


+4 CONCRETE CAP LABLE	NTS	
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DUILDUECN 12	DATE/REV	NOTES

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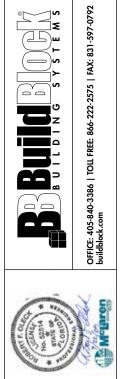


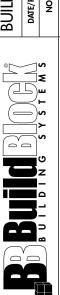
| BUILDDECK 12" +4" CONCRETE CAP CHART

DATE/REV SCALE NTS DETAIL SHEET				NOTES	
	DETAIL SHEET	NTS	SCALE	DATE/REV	

CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.

BuildDeck - 12" Deck (+ 2" EPS Riser) With 2" Concrete Cap	8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36	Live Load	79 65 52 43 34 26 19 13 7 X X X X X X X X X X X X X X X X X X	14 7 X X X X X X X	162 133 110 90 74 60 48 37 28 19 13 7 X X X X X X X	189 156 129 107 88 72 58 46 36 27 20 12 6 X X X X X							
" De	14 15		196 163	332 280	446 359	524 421					#3 rebar, 4 ft each end @ 5" O.C.	12" x 12" Grid #4 Steel	01000
12	13		237 1	394 3	560 4		isd 00	ksi	Jsc	.80	4 ft ea	Grid #	
CK -	12		288 2	473 3	2 289	48 6	F'c = 4000 psi	F'y = 60 ksi	DL = 15 psf	Def < L/480	rebar,	" x 12"	1
Det	1		355 2	574 4		1358 1108 848 661	Fί	Ē	П	۵	#3	12	
	10		442 3	708 5	015 8	358 1							
B	6		561 4		1622 1268 1015 828	691 1.	trength		ncl.		Beam		
	8		726 5	1140 888	622 12	2156 1691	Yield St	‡	³d Ld. I	tion	nent in	벋	
	Span Length (Ft.)	Bar Size Designation	H	#2 1	11 9#	7#	Assumptions: Concrete Design Yield Strength	Steel Yield Strength	Addtl. Applied Dead Ld. Incl.	Long Term Deflection	Stirrup Reinforcement in Beam	Slab Reinforcement	
	Span Lei	Qty. Rein. Steel Rqd.	2	2	2	2	Assumptions:						



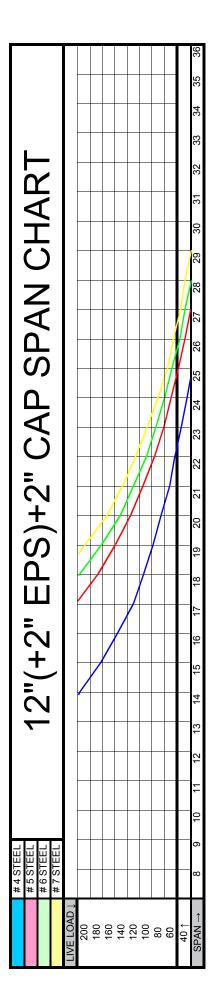


BUILDDECK 12+2 TOP HAT +2" CONCRETE TABLE

DATE/REV	SCALE	NTS	
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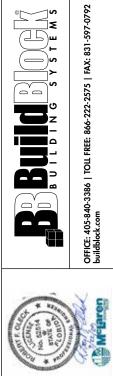


BUILDDECK 12" +2 TOP HAT +2" CONCRETE CAP CHART

DATE/REV	SCALE	NTS	
NOTES			

DETAIL SHEET CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.

			8		BuildDeck - 12"	3Ck	_)e(×	+	_".	PS	X	se	r) V	Deck (+ 2" EPS Riser) With 3" Concrete Cap	3"	ပိ	ncı	ete	Ö	ap						
Span Le	Span Length (Ft.) 8 9 10 11 12 13 14	∞	6	10	11	12	13	14	15	16	17	18	19	20	21	22	15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35	24.	5 2	6	7 28	3 26	30	31	32	33	34	35	36
Qty. Rein. Steel Rqd.	Bar Size Designation														Live	Live Load	р												
2	#	783	603	476	381	308	252	208	172	142	118	26	80	9	23	41	32 2	24 1	16 10	4 (×	×	×	×	×	×	×	×	×
2	9#	1237	696	767 621		510	425	356	301	256	218	187	161	138	116	96	9 8/	63 50	0 39	9 29	21	13	9	×	×	×	×	×	×
2	9#	1771	1771 1384 1107 902 747	1107	902	747	979	530	433	353	290	239	198	164	136	113	93 7	76 61	1 49	38	28	20	13	2	×	×	×	×	×
2	L #	2369 1857 1491 1220 1013 798 634	1857	1491	1220	1013	798	634	510	415	339	280	232	193	161	134 1	111 92	-	76 62	2 49	33	29	21	14	8	×	×	×	×
Assumptions:	Assumptions: Concrete Design Yield Strength	n Yield ;	Streng	th		F'c = 4	F'c = 4000 psi																						
	Steel Yield Strength	ngth				F'y = 60 ksi	0 ksi																						
	Addtl. Applied Dead Ld. Incl.	Dead Ld.	. Incl.			DL = 15 psf	5 psf																						
	Long Term Deflection	ection				Def < L/480	./480																						
	Stirrup Reinforcement in Beam	ement is	n Bean	٦		#3 reb	#3 rebar, 4 ft each	each e	end @ 5" O.C.	" O.C.																			
	Slab Reinforcement	nent				12" x 1	2" Gric	12" x 12" Grid #4 Steel	ē																				
	ALL TABLES PROVIDED FOR ESTIMATION PURPOSES ONLY. ALL DESIGNS MUST BE REVIEWED AND APPROVED BY THE PROJECT SPECIFIC ENGINEER OF RECORD.	3LES PI	ROVID	ED FC	R EST	IMATI	ON PU	RPOS	S ON	Y. AL	L DESI	GNS N	IUST B	E REV	IEWED	AND A	PPRO	/ED B	/ THE	PROJE(T SPE	CIFIC	ENGINE	ER OF	: RECO	RD.			

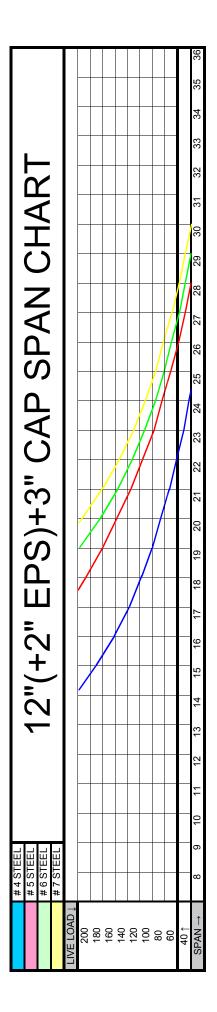




BUILDDECK 12" +2" EPS TOP HAT +3" CONCRETE CAP TABLE

 DATE/REV	SCALE	E NTS	
NOTES			

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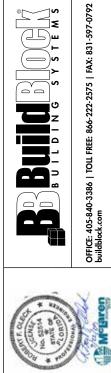
BUILDDECK 12" +2" EPS TOP HAT +3" CONCRETE CAP CHART

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DET/	SCALE NTS	SCALE				DATE/REV	
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CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.

TAIL SHEET

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	36		×	×	×	×							
	15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36		×	×	×	×							
	34		×	×	×	×							
	33		×	×	×	8							8
	32		×	×	9	15							RECO
	31		×	2	13	23							ER OF
dı	30		×	12	21	31							NGINE
Ce	29		×	20	30	41							IFIC E
ete	28		×	29	39	52							SPEC
cre	27		×	38	20	64							JJECT
)on	26		9	20	63	62							IE PRO
Deck (+ 2" EPS Riser) With 4" Concrete Cap	25		13	63	77	92							BYTH
:h 4	24		21	22	94	115							ROVED
Wit	23	aq	30	89	114	137							APPR
۱) ۱	22	Live Load	41	105	138	164							D AND
ise	21	Li	52	124	165	196							VIEWE
SR	20		99	145	198	233							BE RE
ΞPξ	19		82	170	238	280							AUST I
2" E	18		100	198	286	336							GNS
+ 2	17		123	232	346	407							L DES
sk (16		149	272	418	495							Y. AL
)ec	15		180	321	487	809							S ONL
]7	14		219	381	571	754					.0.C		RPOSE
BuildDeck - 12"	13		268	454	674						ld @ 5'	<u>ө</u>	N PUF
Ck	12		328	547	908	1099					ach er	12" x 12" Grid #4 Steel	IMATIC
ID e	11		406	999	975	1324	30 ps	ksi (bsf 5	/480	r, 4 fte	2" Grid	R EST
	10		208	824	197	1619	'c = 40	F'y = 60 ksi	JL = 15	Def < L/480	3 reba	2" × 12	ED FO
8	6	I	949	1037	1497	2018	Strenc F	т.	Incl.		ι Bea#	_	OVIDE
	8	!	839	1334 1	1916 1497 1197 975 806	2576 2018 1619 1324 1099 925	Yield 5	gth	ad Ld.	ction	ment ir	ent	ES PF
	Span Length (Ft.) 8 9 10 11 12 13 14	Bar Size Designation	#4	#2	9#	2	Soncrete Design	Steel Yield Strength	Addtl. Applied Dead Ld. Incl. DL = 15 psf	Long Term Deflection	Stirrup Reinforcement in Bea #3 rebar, 4 ft each end @ 5" O.C	Slab Reinforcement	ALL TABLES PROVIDED FOR ESTIMATION PURPOSES ONLY. ALL DESIGNS MUST BE REVIEWED AND APPROVED BY THE PROJECT SPECIFIC ENGINEER OF RECORD
	Span Len	Qty. Rein. Steel Rqd.	2	2	2	2	Assumptions: Concrete Design Yield Strenc F'c = 4000 ps	(U)	⋖.	_	(J)	(J)	



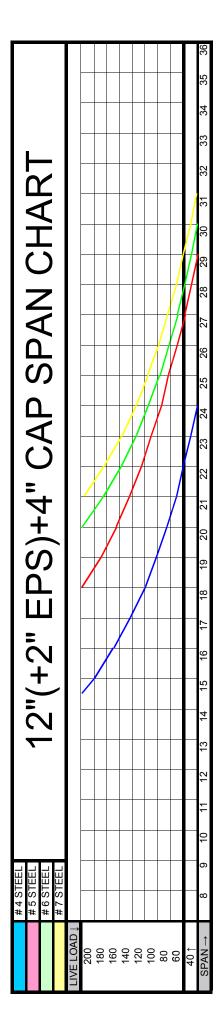


BUILDDECK 12" +2" EPS TOP HAT +4" CONCRETE CAP TABLE

DATE/REV	SCALE	NTS	_
NOTES			

DETAIL SHEET

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BUILDDECK 12" +2" EPS TOP HAT +4" CONCRETE CAP CHART

		· ·	! -	i	; : : :		
	DATE/REV			SCALE	NTS		DETAIL SHEET
	NOTES						
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CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.

	Bu	BuildDeck		ec Dec	- 12" Deck (+ 4" EPS Riser) with 4" Concrete Cap	t" EP	S Rise	ər) wit	h 4" C	oncr	ete Ca	ap		
Span Length	22	87	29	30	31	32	33	34	35	36	37	88	39	40
Bottom Reinf. Requirements						Allow	Allowable Live Load (psf)	ve Loac	(psf)					
	130	115	100	06	9	40	×	×	×	×	×	×	×	×
							Required C	Required Camber (in)						
(3) # 6	0	9.0	0.5	0.5	1	1	N/A	N/A	N/A	N/A	N/A	W/A	N/A	N/A
						0,	Stirrup End Distance (ft)	Distance (ft	(
	7	7	7	7	9	4	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	195	175	160	145	125	06	22	30	×	×	×	×	×	×
							Required (Required Camber (in)						
(2) # 2	0	0.5	0.5	0.5	0.5	1	1	1	×	×	×	×	×	×
`						0,	Stirrup End Distance (ft)	Distance (ft	(
	8.5	8.5	6	6	6	8	9	3.5	N/A	N/A	N/A	N/A	N/A	N/A
	597	240	215	195	180	155	115	08	20	22	×	X	×	×
							Required (Required Camber (in)						
(2) #8	0	0.5	0.5	0.5	0.5	0.5	1	1	1	1	N/A	N/A	N/A	N/A
						3	Stirrup End Distance (ft)	Distance (ft	(
	9.5	10	10	10	10.5	10	9.2	8.5	6.5	1	N/A	N/A	N/A	N/A

ASSUMPTIONS:

fc = 4,000 psi

60 ksi

Fy =

DL = 15 psf

ADDITIONAL NOTES:

1.) Long term deflection kept less than L/480

2.) Required Stirrups: #3 @ 9" O.C. Distances Shown in Table.

3.) Required Slab Reinforcement #5 @ 12" O.C. Parallel to BuildDeck Span, #3 @ 12" O.C. Perpendicular to BuildDeck Span

Stirrup End Distance

4.) Required Lap Splice Lengths #6 = 38", #7 = 54", #8 = 62"

Stirrup End Distance



BUILDDECK 12" +4" EPS TOP HAT +4" CONCRETE CAP TABLE

NOTES

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	Bu	BuildDeck		- 12" Deck (+ 6" EPS Riser) with 4" Concrete Cap	;k (+ (3" EP	S Rise	ər) wit	h 4" C	Soncre	ete Ca	ap		
Span Length	27	28	29	30	31	32	33	34	35	36	28	88	39	40
Bottom Reinf. Requirements						Allow	Allowable Live Load (psf)	ve Loac	(psf)					
	155	135	120	110	92	82	75	22	30	×	×	×	×	×
							Required (Required Camber (in)						
(2) # 6	0	0	0	0.5	9.0	0.5	1	1	1	N/A	N/A	N/A	N/A	N/A
						5	Stirrup End Distance (ft)	Distance (ft	(
	7	7	7	7	2	7	6.5	5.5	3	N/A	N/A	N/A	N/A	N/A
	240	215	195	175	160	145	130	110	75	45	70	×	×	×
							Required (Required Camber (in)						
(2) # 7	0	0	0	0.5	0.5	0.5	0.5	1	1	1	1	N/A	N/A	N/A
							Stirrup End Distance (ft)	Distance (ft	(
	6	6	6	6	6	9.5	9.5	6	7.5	5.5	1	N/A	N/A	N/A
	332	302	275	245	225	202	190	175	140	100	0/	40	×	×
,							Required (Required Camber (in)						
(2) #8	0	0	0	0.5	0.5	0.5	0.5	0.5	1	1	1	1	N/A	N/A
						3	Stirrup End Distance (ft)	Distance (ft	(
	10	10	10.5	10.5	10.5	11	11	11	10.5	9.5	8.5	9	N/A	N/A

ASSUMPTIONS:

f'c = 4,000 psi

DL = 15 psf

Fy = 60 ksi

ADDITIONAL NOTES:

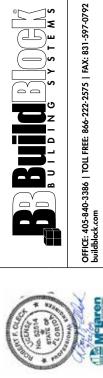
1.) Long term deflection kept less than L/480

2.) Required Stirrups: #3 @ 9" O.C. Distances Shown in Table.

3.) Required Slab Reinforcement #5 @ 12" O.C. Parallel to BuildDeck Span, #3 @ 12" O.C. Perpendicular to BuildDeck Span

4.) Required Lap Splice Lengths #6 = 38", #7 = 54", #8 = 62"

Stirrup End Distance -- Deck Span --Stirrup End Distance





BUILDDECK 12" +6" EPS TOP HAT +4" CONCRETE CAP TABLE

·	DATE/REV		SCALE NTS	NTS		DETAIL SHEET	ا بيز
	NOTES				•		
	CONSTRUCTION SHALL BE IN ACCORDAN SUBJECT TO CHANGE WITHOUT NOTICE.	N ACCORDANC IOUT NOTICE.	E WITH AI	CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.	9	Ī	

	B	BuildDeck	<mark>ck</mark> - 12	⊵" Deα	k (+ {	3" EP	- 12" Deck (+ 8" EPS Riser) with 4" Concrete Cap	ər) wit	h 4" (Soncr	ete Ca	ар		
Span Length	27	28	59	30	31	32	33	34	32	98	37	88	68	40
Bottom Reinf. Requirements						Allow	Allowable Live Load (psf)	ve Loac	(bst)					
	175	155	140	125	110	100	06	80	20	09	40	×	×	×
							Required (Required Camber (in)						
(5) # 6	0	0	0	0	0	0	0.5	0.5	1	1	1	N/A	N/A	N/A
						,	Stirrup End Distance (ft)	Distance (ft	(:					
	7	7	7	7	7	7	7	6.5	6.5	9:2	2	N/A	N/A	N/A
	270	245	220	200	185	165	150	135	125	115	06	09	30	×
							Required (Required Camber (in)						
(2) # 2	0	0	0	0	0	0.5	0.5	0.5	5.0	1	1	1	1	N/A
						,	Stirrup End Distance (ft)	Distance (ft	(:					
	6	6	6	6	9.2	9.5	9.2	9.5	6.5	5'6	6	2	4	N/A
	380	320	320	290	592	245	225	202	190	175	155	120	58	65
							Required (Required Camber (in)						
(2) #8	0	0	0	0	0	0.5	0.5	0.5	5.0	1	1	1	1	1
						٠,	Stirrup End Distance (ft)	Distance (ft	(:					
	10	10	10.5	10.5	10.5	11	11	11	11.5	11.5	11.5	10.5	6.5	8.5
														Ī

ASSUMPTIONS:

ADDITIONAL NOTES:

1.) Long term deflection kept less than L/480 $\,$ f'c = 4,000 psi

2.) Required Stirrups: #3 @ 9" O.C. Distances Shown in Table.

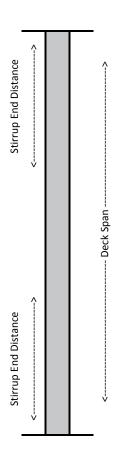
3.) Required Slab Reinforcement #5 @ 12" O.C. Parallel to BuildDeck Span, #3 @ 12" O.C. Perpendicular to BuildDeck Span

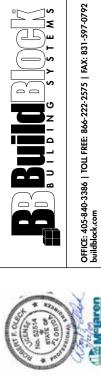
4.) Required Lap Splice Lengths #6 = 38", #7 = 54", #8 = 62"

DL = 15 psf

60 ksi

Fy =







BUILDDECK 12" +8" EPS TOP HAT +4" CONCRETE CAP TABLE

DATE/REV SCALE NTS NOTES CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES. ALL DRAWINGS ARE SUBJECT TO CHANGE WITHOUT NOTICE.	DETAIL SHEET		1-/
DATE/REV NOTES CONSTRUCTIC SUBJECT TO C	SCALE NTS		ON SHALL BE IN ACCORDANCE WITH ALL APPLICABLE LOCAL AND NATIONAL CODES, ALL DRAWINGS ARE HANGE WITHOUT NOTICE.
	DATE/REV	NOTES	CONSTRUCTIC SUBJECT TO C



NORTH AMERICAN MANUFACTURING FACILITIES

BuildBlock Building Systems has fourteen manufacturing facilities across North America and plans to add locations for the next several years. This means we have the manufacturing capacity to meet your ICF needs now and in the future. Shorter shipping distances mean lower freight costs for you and your customers.

BuildBlock continually develops new products and technologies solving problems and meeting needs in residential, commercial, industrial, and institutional construction. We innovate with the goal of creating cost-effective techniques and products for our customers.

BuildBlock partners have facilities around the world to meet your needs including the Philippines, Cyprus, and Egypt and continue to expand. Choosing BuildBlock isn't just about choosing the best ICF block on the market, it's about finding a partner with a strong commitment to our customers, our business partners, and our industry.

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MISSION

To harmoniously use the extraordinary gifts and talents of our distributors and dealers to fulfill the goals and dreams of millions of people who want to build better structures as reflected by our motto: "Build it once. Build it for life."

To manufacture one of the most affordable and highest quality Insulating Concrete Forms available in the world today.

To build greatness by providing the resources and services needed for building successful ICF businesses and sustainable

To build an enduring, profitable company while conducting business with Godly character, fairness and integrity.

VISION

We envision a world where BuildBlock ICF technology delivers energy-efficient, safe, healthy, comfortable and sustainable ICF homes and buildings to millions of people worldwide through the uncompromising integrity of BuildBlock's team of distributors, dealers and customers.

VALUES

INTEGRITY - We strive to balance the best interests of our distributors, dealers, customers, employees, and investors in an environment of Godly character and honesty.

EDUCATION - We seek to educate the public on the valuable benefits of ICF structures while recognizing that in order to expand the industry, we must educate installers, architects, and engineers in ICF best practices.

CUSTOMER SATISFACTION – We commit to building a team of employees that is inspired, empowered, and driven to meet the ever-changing needs of our distributors, dealers, and customers while we seek to distinguish ourselves in the marketplace by delivering exceptional customer satisfaction.

INNOVATION - We value and invest heavily in innovation while continually expanding our product line through the development of technologically advanced products.

QUALITY – We commit to producing the finest quality products. We stand by the belief that our brand embodies quality, consistency, user satisfaction, and service.

PROFITABILITY – We commit to the strong work ethic and financial prudence necessary to deliver financial results for our business partners and investors and to ensure a long-term profitable relationship.

EMPOWERMENT - We dedicate ourselves to empowering people to improve and enrich their lives and the world around them.

